



General Services Agency
Capital Projects
COUNTY OF TULARE
2637 W BURREL AVE., SUITE 200
VISALIA, CA 93291



January 05, 2021

ADDENDUM NO. 3
COUNTY OF TULARE – Sheriff and Fire Dispatch Project

Any Addenda issued by the Owner or Owner's Representative during the time of bidding are to be considered in the Bid and will become a part of the Agreement between Contractor and Owner. Bidders shall acknowledge receipt of all Addenda on the Bid Form in the space provided.

ITEM 1: Soils Report

Attached is the Soils Report that was referenced in the Plans. The report is for informational purposes only. The County does not view this report and the issuance thereof as a material change to the project. Therefore, the bid due date remains the same as in the Project Specifications.

Kyle Taylor
Capital Projects Coordinator III

End of Addendum No. 3

**SOIL INVESTIGATION FOR
TULARE AKERS PROFESSIONAL
CENTER NEW COMMUNICATION TOWER
AKERS STREET,
VISALIA, CALIFORNIA**

Submitted to:
Capital Projects

May 29, 2020

Submitted by:
CTL- INC.

May 29, 2020
File No. E2755-20

Mr. Kyle Taylor
Capital Projects
5953 south Mooney Blvd.,
Visalia, Ca. 93277

**SUBJECT: Soil Investigation for proposed Tulare Akers Professional Center
New Communication Tower, Akers Street, Visalia, California.**

Gentlemen:

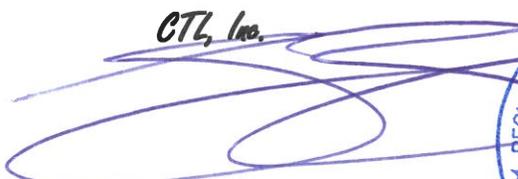
At your authorization and request, we have performed a Soil Investigation for Tulare Akers Professional Center New Communication Tower, Akers Street, Visalia, California.

The accompanying report presents the results of our Soil Investigation for the above referenced project. The report describes our study, findings, conclusions, and recommendations for use in design by the project consultants. It is the client's responsibility to see that all parties to the project, including the designer, contractor, subcontractors, etc., are made aware of this report in its entirety, including the Additional Services and Limitations sections.

We appreciate the opportunity to be of service. If you have questions regarding the information contained in this report, please contact us.

Respectfully submitted,

CTL, Inc.



Santiago Espinoza
Managing Engineer
RCE No. 83299

SE:rc

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APPENDIX

APPENDIX A	Suggested Earthwork Specifications
APPENDIX B	Testhole Boring Logs
APPENDIX C	Laboratory Findings
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The first part of the document discusses the importance of maintaining accurate records of all transactions. It emphasizes that every sale, purchase, and transfer must be properly documented to ensure compliance with tax laws. The text also highlights the need for regular audits and reviews to identify any discrepancies or errors in the accounting process.

Furthermore, it outlines the various methods used for recording financial data, including the use of journals, ledgers, and trial balances. The document provides detailed instructions on how to properly format and maintain these records, ensuring that they are clear, concise, and easy to understand.

In addition, the text covers the importance of keeping up-to-date with changes in tax regulations and accounting standards. It advises that businesses should consult with a qualified professional to ensure that their records and reporting are always in compliance with the latest requirements.

The second part of the document focuses on the practical aspects of accounting, such as how to calculate and report income, expenses, and profits. It provides step-by-step instructions for determining the taxable income of a business and for calculating the amount of taxes owed. The text also discusses the various deductions and credits that can be claimed to reduce the tax liability.

Moreover, it covers the process of preparing and filing tax returns, including the necessary forms and documentation. The document provides a comprehensive overview of the tax filing process, from gathering the necessary information to submitting the return to the appropriate tax authority.

Finally, the text discusses the importance of maintaining accurate records for long-term financial planning and analysis. It explains how these records can be used to track the performance of a business over time and to identify areas for improvement. The document concludes by emphasizing the value of professional accounting services in ensuring that a business's financial records are accurate and compliant with all applicable laws and regulations.

**SOILS INVESTIGATION
FOR THE NEW COMMUNICATIONS TOWER
TULARE COUNTY AKERS PROFESSIONAL CENTER
5300 WEST TULARE STREET
VISALIA, CALIFORNIA**

INTRODUCTION

This report presents the results of a Soils Investigation for the Proposed Communications Tower at the Tulare County Akers Professional Center, 5300 West Tulare Street in Visalia, California. The purpose of the investigation was to explore and evaluate the subsurface conditions, and to make recommendations for the site preparation procedures and foundation parameters. This report includes the field and laboratory investigation data and presents geotechnical conclusions and recommendations. This report is based upon data obtained from one soil boring and laboratory tests performed on samples obtained from the boring and bulk samples.

SITE LOCATION AND DESCRIPTION

The project is located at 5300 West Tulare Street in Visalia, California. The latitude is 36.3228 degrees and the longitude is -119.3514 degrees at the approximate center of the site. A Site Location Map is presented in Appendix C. At the time of the investigation, the proposed construction area lies within an existing solar parking structure. The description of the site is based on visual observations made during our field investigation on May 22, 2020.

PROPOSED DEVELOPMENT

Based on information obtained, the proposed Communication Tower will be self-supported. Structural loads for the Tower will be supported on a pier footing. No detailed structural information was provided at this time. Appurtenant construction will include underground utilities.

SOIL AND GROUNDWATER CONDITIONS

The subsurface soils encountered generally consist of sandy silt, sand and sandy clay. The upper surface is generally medium dense sandy silt to 13 feet below grade (BG) underlain by medium dense sand to a depth of 18 feet BG, the clayey sand was underlain by sandy silt, sand and sandy clay to a depth of 18 feet BG. The sand was underlain by dense to very dense silty sand, clayey sand and sandy clay to 36.5 feet BG, maximum depth explored.

Groundwater was not encountered during our field exploration during and after drilling. It should be noted that groundwater level fluctuates due to variations in precipitation, land use, irrigation, and other factors. The evaluation of these factors is beyond our scope of services.

The soil profile described above is generalized, therefore, the reader is advised to consult the Logs of Borings in Appendix A for soil conditions at specific locations or depths. Care should be exercised in interpolating or extrapolating subsurface conditions beyond the boring locations.

Locations of our exploratory borings, shown on the Boring Location Maps in Appendix C were determined with a measuring wheel from features shown on the Site Plan provided for our use. Hence, the accuracy of the boring locations can be implied only to the degree that this method warrants. Surface elevations at the boring locations were not measured.

The first part of the document discusses the importance of maintaining accurate records of all transactions. It emphasizes that every entry, no matter how small, should be recorded to ensure the integrity of the financial data. This includes not only sales and purchases but also expenses, income, and any other financial activities. The text explains that proper record-keeping is essential for identifying trends, managing cash flow, and preparing for tax obligations.

Next, the document addresses the need for regular reconciliation. It states that comparing the company's internal records with bank statements and other external sources is crucial for catching errors early. This process helps to identify discrepancies, such as missing transactions or incorrect amounts, and allows for prompt correction. Regular reconciliation also provides a clear picture of the company's current financial position and helps to prevent fraud or mismanagement.

The document then discusses the importance of budgeting and forecasting. It explains that setting a budget allows a company to plan its future financial needs and allocate resources effectively. By forecasting income and expenses, management can anticipate potential challenges and make informed decisions to avoid them. The text also notes that budgeting is a key tool for measuring performance and staying on track with financial goals.

Finally, the document emphasizes the importance of transparency and communication. It states that all financial information should be shared with relevant stakeholders, including management, investors, and tax authorities. Clear communication ensures that everyone is on the same page and can make decisions based on accurate and complete information. The document also highlights the importance of maintaining confidentiality and security of financial data to protect the company's interests.

CONCLUSIONS AND RECOMMENDATIONS

1.0 General

Based on field data and engineering analyses, the site is suitable for the proposed construction provided the site is graded in accordance with 2019 California Building Code and that our recommendations are incorporated into the project design and are followed throughout the construction.

Expansive soil was encountered within the near surface soils at the site. The subject site is not near any active known fault, and surface rupture does not apply. Groundwater was not encountered in our exploration boring. Therefore, there is a low liquefaction potential and lateral spreading to occur at the site. No special mitigation is required.

Detailed site grading and foundation design recommendations are presented in the following sections.

2.0 Site Preparation

2.1 Clearing: Prior to earthwork operations, the area to be developed should be stripped of vegetation, organic topsoil, undocumented fills and cleared of surface and subsurface obstructions and miscellaneous debris from the proposed construction areas. **We estimate the depth of clearing to be approximately four to nine inches.** Deeper clearing may be required in localized areas. The actual depth of clearing should be reviewed by CTL. Since no grading is involved with the construction of the tower, no further site preparation is required.

2.2 Compaction: The scarified subgrade and any subsequent fill placed at the site should be moisture conditioned to within 0 to 4 percent over the optimum moisture content and compacted to at least 90 percent of maximum dry density as determined by ASTM Test Method D1557. Aggregate Base should be compacted to 95 percent.

2.3 Engineered Fill: Fill should consist of select material. Native soils, free from organic, vegetation and rocks or cobbles larger than one inch and have an expansion index less than 20, may be used as Engineered Fill. Fill with an organic content higher than 3 percent by dry weight should not be used and removed from the site or used in non-structural areas as approved by the owner. Import material if required, must be reviewed by CTL prior to transport to the site. Import material should conform to the following criteria:

ENGINEERED FILL REQUIREMENTS	
Maximum Expansion Index	20
Maximum Particle Size (inch)	1
Percentage Passing #200 Sieve	12-70
Maximum Water-Soluble Sulfate in Soil, percent by dry weight	0.2

2.4 Fill Placement: Fill material should be moisture-conditioned to within 0 to 4 percent above the optimum moisture content prior to compaction. Fill material with excessive moisture should be allowed to dry prior to compaction or be mixed with dry soil to bring the fill to a workable moisture content. Fill should be placed in level lifts not exceeding a loose, uncompacted thickness of eight inches, and compacted as engineered fill.

2.5 Utility Trench Backfill: The underground utilities should be installed according to the manufacturer's recommendations. However, where no manufacturer's recommendations are available, underground utilities should be installed as described below. Underground utility lines should have no less than 18 inches of cover. A minimum of six inches of compacted sand bedding under the pipe, and a pipe envelop extending six inches above the pipe, should be provided. The remaining backfill material should consist of Engineered Fill as described previously in this report. Utility trench backfill should be moisture conditioned and compacted to 95 percent in the upper 2 feet in structural pavement areas and 90 percent below 2 feet from the top of final grade

3.0 Foundation Recommendations

3.1 Drilled Piers: The proposed structures may be supported on straight shaft, cast-in-place, concrete piers with the supporting capacity derived from friction and end bearing. The drilled piers should be a minimum of 24 inches in diameter. A structural engineer should design the piers to resist the moment, shear and axial loads.

These piers may be designed using the allowable end bearing capacity of 3,000 pounds per square foot (psf) and allowable skin resistance of 300 psf. The allowable pier support capacity is for combined dead and sustained live loads and may be increase by one-third (1/3) when including transient live loads due to wind or seismic loading. To calculate the uplift resistance, the allowable friction resistance is compression listed above may be multiplied by a reduction factor of 0.6, plus the pier weight. Settlement is expected to be due to substantially to elastic compression of the foundation materials and should be essentially complete following application of the design loads. The total settlement to the cast-in-place piers is expected to be on the order of ½ of an inch.

Resistance to lateral loads may be determined by using the “Pole Formula” given in Section 1807.3.2.1 of the California Building Code. For this method, we recommend the lateral soil bearing pressure of 300 pounds per square foot per foot of embedment to be used to establish the required embedment depth (maximum of 3,000 psf). It is recommended that the piers have a minimum embedment depth of 10 feet below grade. The design value may be increased to twice the above value of the structure which will not be adversely affected by a half inch lateral deflection at the ground surface due to short-term lateral loads.

Loose soils at the bottom of the drilled piers should be removed to the extent possible by a cleanout bucket or other pier cleaning equipment. A representative of CTL should be present at the site during pier drilling and concrete placement operation to establish substantial conformance with the design concepts and specification requirements.

The structural engineer should provide the structural specifications for the cast-in-place drilled hole foundation. Concrete should be placed in the drilled shaft as

soon as possible following the drilling. If required, sand layers require temporary casing to support the excavations during construction. The casing should be slowly removed from the shaft excavation during placement of concrete while ensuring the casing is not raised above the level of the concrete during the shaft construction. As an alternate to temporary casing, it may be possible to utilize a drilling slurry for temporary support of the foundation if unstable sidewall conditions occur.

3.2 Seismic Design Parameters: The proposed structure should be designed with construction specifications and structure properties to withstand the anticipated or probable effects of seismic ground motion, if a seismic event was to occur. The approximate center of the project site is at a latitude of 36.3228 ° and longitude of -119.3514°. Probabilistic values of ground motion corresponding to various levels of seismic hazards have been established by CGS and USGS based on ASCE 7-16. Based on the new procedure in Section 1613 of 2019 California Building Code (CBC), the seismic design parameters are provided as follows:

SEISMIC DESIGN PARAMETERS 2019 CBC		
Property	Symbol	Value
Occupancy Category	-	I, II or III
Site Class	-	D
Mapped MCE Acceleration at Short Periods	S _s	0.567
Mapped MCE Acceleration at 1-Second Periods	S ₁	0.223
Site Coefficient	F _a	1.346
Adjusted MCE Spectral Response Acceleration Parameter	S _{MS}	0.764
Design Spectral Acceleration Parameter	S _{DS}	0.509
Mapped Maximum Considered Earthquake MCE _G	PGA	0.247
Maximum Considered Earthquake MCE _G adjusted for site effects	PGA _M	0.334

4.0 Lateral Earth Pressure and Frictional Resistance

For structures subject to lateral pressures from native soils and backfill at the Site, the following values are recommended:

Lateral Earth Pressures	
Lateral Pressure and Condition	Equivalent Fluid Pressure, pcf
Active case, drained	45
At-rest case, drained	60
Passive case, drained	360

Design values assume level, drained granular backfill. Pressures due to surcharge loads from adjacent footings, traffic, etc., should be analyzed separately. The upper one foot of soil of the adjacent grade should not be used in the passive pressure computation. A coefficient of friction of 0.35 may be used between subgrade soil and concrete footings. Vertical soil loads may be calculated based on soil bulk density of 120 pounds per cubic foot.

The foregoing equivalent fluid pressures and fractional coefficients represent ultimate soil values, and a safety factor consistent with design conditions should be included. A minimum safety factor of 1.5 against lateral sliding is recommended if the sliding is resisted only by frictional resistance. When combined passive and frictional resistance is used, we recommend a minimum safety factor of 2.0. For lateral stability against seismic loading, we recommend a minimum safety factor of 1.1.

5.0 Additional Services

The review of plans and specifications, construction consultation, and field observation by CTL, Inc. are an integral part of the conclusions and recommendations made in this report. These are vital elements and extensions of this geotechnical engineering investigation. We recommend that following the development of construction plans and specifications, those portions of the contract drawings and specifications that pertain to earthwork be made available to CTL, Inc.

to verify that they are consistent with our recommendations contained in this report. We recommend that CTL, Inc. be retained to provide geotechnical consultation and construction testing services during site preparation and grading phases of the project. This would include observation and testing of the earthwork.

CHANGED CONDITIONS AND LIMITATIONS

Findings of this report are valid as of the present. However, changes in proposed construction such as structure type, design loads, and location may invalidate the report. Also, site conditions and applicable standards may change. Therefore, this report should be reviewed to determine its applicability considering changed conditions or after a substantial lapse of time between the preparation of our report and the start of work at the site (two years or more). The analyses and recommendations submitted in this report are based upon the data obtained from the exploratory borings performed. The samples obtained and tested, and the observations made, are assumed to be representative of the site soils. The report does not reflect variations which may occur between borings. The validity of the recommendations contained in this report is also dependent upon the prescribed testing and observation program during the site preparation and construction phases. Our firm assumes no responsibility for construction compliance with these design concepts and recommendations unless we have been retained to perform observation and review during site preparation, grading, and foundation/slab construction. CTL, Inc. has prepared this report for the exclusive use of the client noted on the cover page and the project design consultants. The report has been prepared in accordance with generally accepted practices by reputable geotechnical engineers practicing in this or a similar locality at the time the report was written. No other warranties, either expressed or implied, are made as to the professional advice provided under the terms of this agreement and included in this report.

The first part of the document discusses the importance of maintaining accurate records of all transactions. It emphasizes that every entry, no matter how small, should be recorded to ensure the integrity of the financial data. This includes not only sales and purchases but also expenses, income, and any other financial activities.

The second part of the document provides a detailed breakdown of the accounting process. It starts with the identification of the accounting period, followed by the collection and classification of data. The next steps involve the recording of transactions in the journal, the posting of these transactions to the ledger, and the preparation of financial statements.

The third part of the document focuses on the analysis and interpretation of the financial statements. It explains how to use the balance sheet, income statement, and cash flow statement to assess the financial health of the organization. It also discusses the importance of comparing the current period's performance with the previous period and with industry benchmarks.

The fourth part of the document addresses the role of the accountant in the organization. It highlights the need for the accountant to be not only a technical expert but also a strategic advisor. This involves providing insights into the financial implications of various business decisions and helping management to make informed choices.

The fifth part of the document discusses the challenges and opportunities in the field of accounting. It notes that while the profession has become increasingly complex due to technological advancements and global economic changes, it also offers significant opportunities for growth and specialization.

The sixth part of the document provides a summary of the key points discussed and offers some final thoughts on the future of accounting. It concludes by emphasizing the importance of continuous learning and professional development in this dynamic field.

TEST BORING LOG LEGEND

DEPTH	SAMPLES	SOIL GROUP	
0'			<p>UNDISTURBED TUBE SAMPLE (2-3/8" INSIDE DIAMETER SPLIT SPOON SAMPLER OR 1-3/8" INSIDE DIAMETER OR STANDARD PENETRATION SAMPLER (SPLIT BARREL SAMPLER)</p>
1'			
2'			
3'			
4'			
5'			
6'			
7'			
8'			
9'			
10'			<p>NO RECOVERY</p>
11'			
12'			
13'			
14'			
15'			
16'			
17'			
18'			
19'			
20'			<p>PARTIAL RECOVERY</p>
21'			
22'			
23'			
24'			
25'			
26'			
27'			
28'			
29'			
30'			

0'
1'
2'
3'
4'
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6'
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30'

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UNDISTURBED TUBE SAMPLE (2-3/8" INSIDE DIAMETER SPLIT SPOON SAMPLER OR 1-3/8" INSIDE DIAMETER OR STANDARD PENETRATION SAMPLER (SPLIT BARREL SAMPLER)

NO RECOVERY

PARTIAL RECOVERY

STANDARD PENETRATION BLOW COUNTS FOR 6" DRIVE OF SAMPLER USING 140LBS. DROP HAMMER WITH 30" DROP

SMALL DISTURBED SAMPLE COLLECTED FROM TESTHOLE CUTTINGS

LARGE BULK SAMPLE COLLECTED FROM TESTHOLE CUTTINGS

HNU 101 PHOTOIONIZATION ANALYZER FIELD READING IN (PPM)

SOIL SAMPLE NUMBER

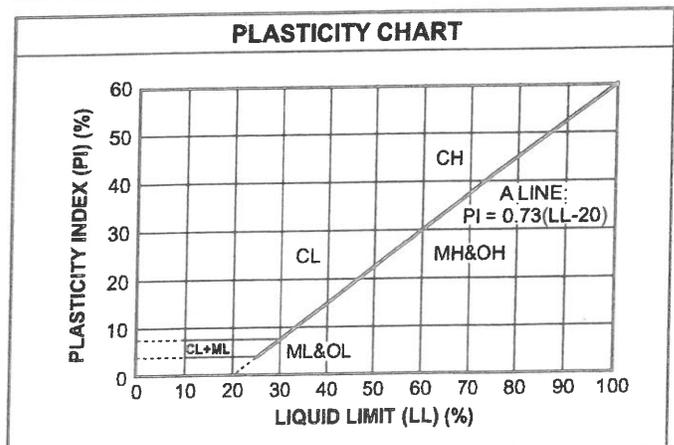
UNIFIED SOIL CLASSIFICATION SYSTEM

UNIFIED SOIL CLASSIFICATION AND SYMBOL CHART		
COARSE-GRAINED SOILS (more than 50% of material is larger than No. 200 sieve size.)		
Clean Gravels (Less than 5% fines)		
GRAVELS More than 50% of coarse fraction larger than No. 4 sieve size		GW Well-graded gravels, gravel-sand mixtures, little or no fines
		GP Poorly-graded gravels, gravel-sand mixtures, little or no fines
	Gravels with fines (More than 12% fines)	
		GM Silty gravels, gravel-sand-silt mixtures
	GC Clayey gravels, gravel-sand-clay mixtures	
Clean Sands (Less than 5% fines)		
SANDS 50% or more of coarse fraction smaller than No. 4 sieve size		SW Well-graded sands, gravelly sands, little or no fines
		SP Poorly graded sands, gravelly sands, little or no fines
	Sands with fines (More than 12% fines)	
		SM Silty sands, sand-silt mixtures
	SC Clayey sands, sand-clay mixtures	
FINE-GRAINED SOILS (50% or more of material is smaller than No. 200 sieve size.)		
SILTS AND CLAYS Liquid limit less than 50%		ML Inorganic silts and very fine sands, rock flour, silty of clayey fine sands or clayey silts with slight plasticity
		CL Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays
		OL Organic silts and organic silty clays of low plasticity
SILTS AND CLAYS Liquid limit 50% or greater		MH Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts
		CH Inorganic clays of high plasticity, fat clays
		OH Organic clays of medium to high plasticity, organic silts
	PT Peat and other highly organic soils	

LABORATORY CLASSIFICATION CRITERIA		
GW	$C_u = \frac{D_{60}}{D_{10}}$ greater than 4; $C_c = \frac{D_{30}}{D_{10} \times D_{60}}$ between 1 and 3	
GP	Not meeting all gradation requirements for GW	
GM	Atterberg limits below "A" line or P.I. less than 4	Above "A" line with P.I. between 4 and 7 are borderline cases requiring use of dual symbols
GC	Atterberg limits above "A" line with P.I. greater than 7	
SW	$C_u = \frac{D_{60}}{D_{10}}$ greater than 4; $C_c = \frac{D_{30}}{D_{10} \times D_{60}}$ between 1 and 3	
SP	Not meeting all gradation requirements for GW	
SM	Atterberg limits below "A" line or P.I. less than 4	Limits plotting in shaded zone with P.I. between 4 and 7 are borderline cases requiring use of dual symbols.
SC	Atterberg limits above "A" line with P.I. greater than 7	

Determine percentages of sand and gravel from grain-size curve. Depending on percentage of fines (fraction smaller than No. 200 sieve size), coarse-grained soils are classified as follows:

Less than 5 percent GW, GP, SW, SP
 More than 12 percent GM, GC, SM, SC
 5 to 12 percent Borderline cases requiring dual symbols



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Next, the document addresses the process of reconciling bank statements. It provides a step-by-step guide on how to compare the company's records with the bank's records to identify any discrepancies. Common reasons for differences, such as bank fees, interest, or timing differences, are discussed. The importance of resolving these discrepancies promptly is highlighted to prevent errors from accumulating and affecting the overall financial picture.

The third section focuses on budgeting and financial forecasting. It outlines how to create a realistic budget based on historical data and current market conditions. The text discusses various forecasting techniques, such as trend analysis and ratio analysis, and provides examples of how to use these methods to predict future performance. It stresses that a well-defined budget is crucial for setting financial goals and monitoring progress throughout the year.

Finally, the document covers the importance of regular financial reviews. It explains that periodic assessments of the company's financial health are necessary to identify areas for improvement and make informed decisions. The text suggests conducting monthly or quarterly reviews and involving key management personnel in the process. It also discusses the role of external auditors in providing an independent opinion on the company's financial statements.

APPENDIX C
LABORATORY
SOIL TEST DATA

DIRECT SHEAR TEST

Quick-consolidated, direct shear tests were performed on undisturbed, saturated samples of native materials. These tests provide information on soil shear strength vs. Normal load and are used to determine the angle of internal friction and cohesion of earth materials under essentially "drained" conditions.

SOLUBLE SULFATE TEST

Combined samples from various locations on the site were collected for soluble sulfate tests. Tests were performed on samples taken from the upper five (5) feet to determine the extent to which measures should be taken (if any) to prevent sulfate attack on concrete surfaces exposed to direct contact with soils. The result of the tests show sulfate content in the areas to be tested are less than 0.1% by weight, indicating that special procedures as mentioned above should not be required.

Grain size distributions for samples selected as most representative of sub-soils encountered in our test borings were determined by Sieve Analysis (ASTM Test D422).

PROJECT: New Tower at Akers Professional
Complex, Visalia, Ca.

FILE NO. E2755-20
DATE: 05/28/20

**DIRECT SHEAR TESTS
(UNDISTURBED SAMPLES)**

**MOISTURE CONTENT
% OF DRY WT.**

SAMPLE LOCATION	DEPTH	BEFORE TEST	AFTER TEST	COHESION LBS./SQ.FT.	INTERNAL FRICTION	USCS
B1	6'-6.5'	17.5	26.8	90	30°	ML
B1	11'-11.5'	2.89	10.10	0	32°	SP
B1	25'-25.5'	5.50	12.48	0	30°	SP

PROJECT: Soils investigation
for New Tower at Akers Professional Complex
Visalia, Ca.

FILE NO: E2755-20
Date: 5/26/2020

TABLE 2

CHEMICAL TESTS

SAMPLE I.D.	SULFATE CONTENT (ppm)	CHLORIDE CONTENT (ppm)	pH (Std. units)	MINIMUM RESISTIVITY (ohm-cm)
B1 @ 0-3'	300	120	7.5	2,500

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...the tenth of the ...

...the eleventh of the ...

...the twelfth of the ...

...the thirteenth of the ...

...the fourteenth of the ...

...the fifteenth of the ...

...the sixteenth of the ...

...the seventeenth of the ...

...the eighteenth of the ...

...the nineteenth of the ...

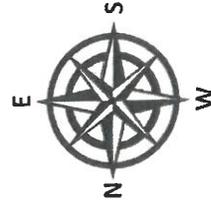
...the twentieth of the ...

...the twenty-first of the ...

...the twenty-second of the ...

Cigna Building Aerial View

Akers Street



LEGEND

 Approximate location of testhole boring

**Tulare Akers Professional Center
New Communication Tower
Akers Street,
Visalia Ca**