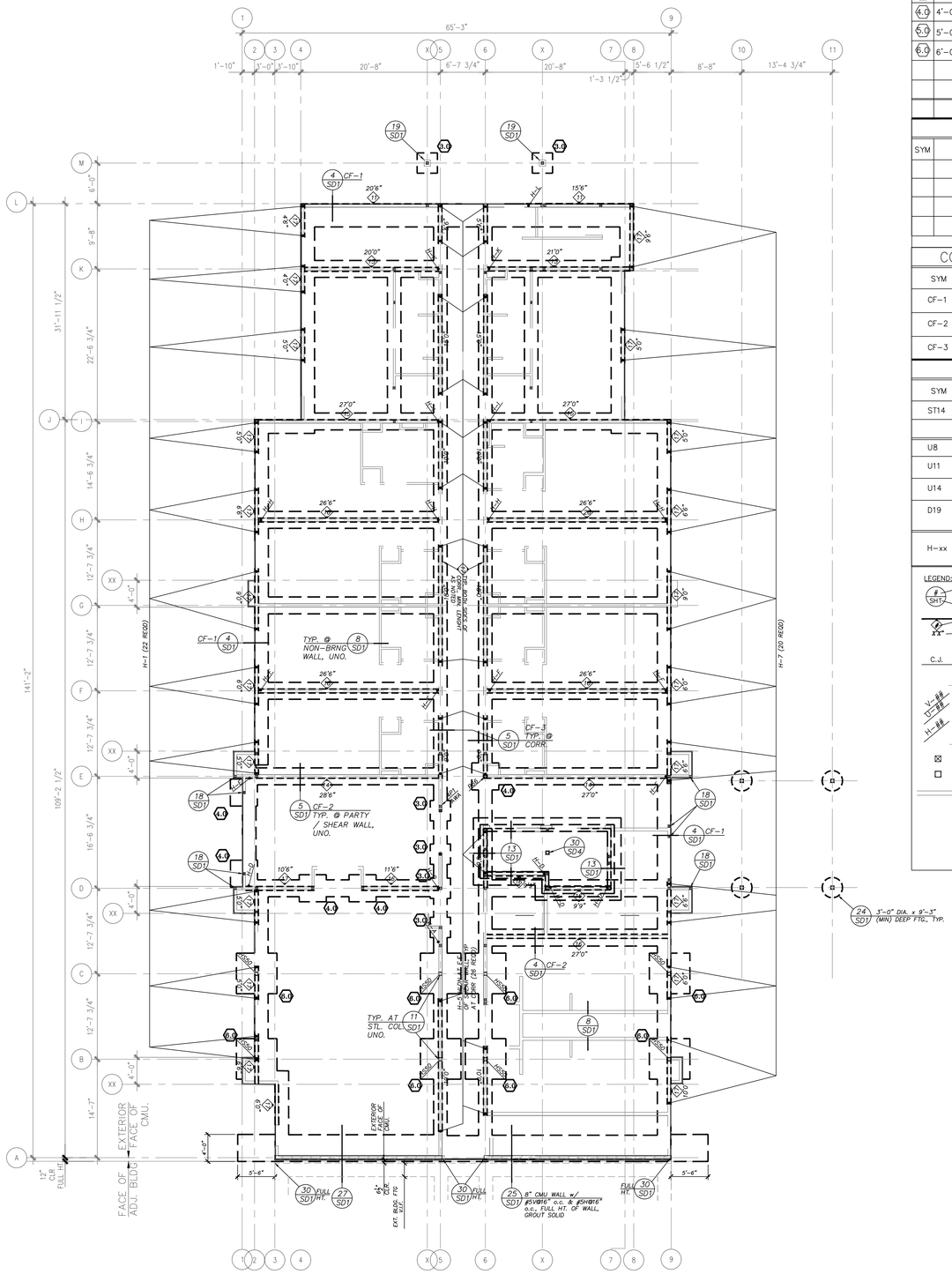

1 FOUNDATION PLAN
SCALE 1/8"=1'-0"

FOOTING SCHEDULE

SYM	SIZE B x D x t	REINFORCING
3.0	3'-0"x3'-0"x12" THICK PAD FTG.	#5 REBAR @ 12" o.c. EACH WAY
4.0	4'-0"x4'-0"x15" THICK PAD FTG.	#5 REBAR @ 12" o.c. EACH WAY
5.0	5'-0"x5'-0"x15" THICK PAD FTG.	#5 REBAR @ 12" o.c. EACH WAY
6.0	6'-0"x6'-0"x18" THICK PAD FTG.	#5 REBAR @ 12" o.c. EACH WAY

GRADE BEAM SCHEDULE

SYM	REINFORCING

CONTINUOUS FOOTING LEGEND

SYM	REINFORCING
CF-1	24"WX24"D w/ 5-#5 T&B & #3 TIES @ 18" o.c. (TYP. EXT. WALL, U.N.O.)
CF-2	30"WX24"D w/ 6-#5 T&B & #3 TIES @ 18" o.c. (TYP. PARTY WALL, U.N.O.)
CF-3	24"WX24"D w/ 5-#5 T&B & #3 TIES @ 18" o.c. (TYP. CORR. WALL, U.N.O.)

HOLDOWN SCHEDULE

SYM	SIZE	DETAIL
ST14	STHD14	18 SD1
U8	HDU8	10 SD1
U11	HDU11	
U14	HDU14	
D19	HD19	
H-xx	SPECIAL PATENTED HOLDOWN SYSTEM SEE SHEETS ATS-1 & ATS-2 HOLDOWN DETAILS FOR ADDITIONAL INFORMATION.	1 SD5

LEGEND:
- 8 / SHT — DETAIL NUMBER / SHEET REFERENCE
- SHEAR WALL TYPE / SHEAR WALL LENGTH
- C.J. — CONTROL JOINT / 7 SD1
- INDICATES HOLDOWN / VERTICAL STRAP, SEE HOLDOWN SCHEDULE FOR ADDITIONAL INFORMATION.
- POST
- STEEL COLUMN
- WALL ABOVE CURRENT FRAMING LEVEL

FOUNDATION NOTES

1. SEE GENERAL NOTES ON SN-1
2. SEE ARCHITECTURAL PLANS FOR DIMENSIONS OF ALL WALL LOCATIONS.
3. PROPERLY LOCATE ALL HOLDOWNS AND CONCRETE EMBEDDED ITEMS AND SECURELY TIE HOLDOWNS IN PLACE PRIOR TO FOUNDATION INSPECTION.
4. GRADING AND FOUNDATION TO BE INSPECTED AND CERTIFIED IN WRITING BY THE SOIL ENGINEER PRIOR TO PLACEMENT OF CONCRETE.
5. EXCAVATIONS SHALL BE MADE AS NEAR AS POSSIBLE TO THE NEAT LINES REQUIRED BY THE SIZE AND SHAPE OF THE STRUCTURE. NO MATERIAL IS TO BE EXCAVATED UNNECESSARILY.
6. VERIFY LOCATION OF UNDERGROUND UTILITIES BEFORE EXCAVATION. NOTIFY ARCHITECT PRIOR TO EXCAVATION IN THE EVENT SUCH UTILITIES ARE ENCOUNTERED.
7. FOR DRAINAGE DETAILS, SUMPS, PITS, DAMPROOFING, TRENCHES, CURBS, EXTERIOR WALKS, UTILITIES, EQUIPMENT DETAILS, STEPS, ETC., SEE DRAWINGS OTHER THAN STRUCTURAL.
8. SLAB CONSTRUCTION AND CONTROL JOINT LOCATIONS SHALL BE APPROVED BY THE ARCHITECT PRIOR TO PLACING ANY CONCRETE SEE DETAILS 7 SD1
9. ALL SILL PLATE ANCHORS SHALL BE 5/8"DIA.x12"A.B. @ 48" O.C. WITH 3"x3"x1/4" PLATE WASHERS TYPICAL UNLESS OTHERWISE NOTED ON SHEARWALL SCHEDULE ON SN-1.
10. PIPE THRU FOOTING PER DETAIL 1 SD1
11. STEPS ON GRADE SHALL BE PER DETAIL 17 SD1
12. PROVIDE SURVEY STAKES PRIOR TO FOUNDATION INSPECTION TO VERIFY LOT LINES.
13. CIVIL ENGINEER TO VERIFY FINISHED FLOOR ELEVATION PRIOR TO CONSTRUCTION AND SUBMIT LETTER TO BUILDING DEPARTMENT.
14. FINISHED FLOOR TO BE 6" MINIMUM ABOVE FINISHED GRADE AND 2" MINIMUM AT WEEP SCREED.
15. U-##, ST-##, H-## — DENOTES WALL HOLDDOWN SYSTEM. SEE HOLDOWN SCHEDULE. COORDINATE LOCATION WITH FRAMING AT LEVEL ABOVE. 10 SD1
    DENOTES EARTHBOUND INC. HOLDOWN SYSTEM. SEE SHEETS ATS-1, ATS-2 & S-5. COORDINATE LOCATION WITH FRAMING AT LEVEL ABOVE.
16. CONTRACTOR TO REVIEW W/ FRAMING CONTRACTOR THE LOCATION OF ALL EMBEDDED ITEMS. CONTRACTOR TO BRING TO THE ARCHITECTS ATTENTION ANY DISCREPANCIES FOUND PRIOR TO START OF CONSTRUCTION.
17. CONTRACTOR TO REVIEW W/ FRAMING CONTRACTOR THE LOCATION OF ALL EMBEDDED ITEMS. CONTRACTOR TO BRING TO THE ARCHITECTS ATTENTION ANY DISCREPANCIES FOUND PRIOR TO START OF CONSTRUCTION.
18. GROUND SURFACE SHALL BE SLOPED AWAY FROM THE BUILDING PAD AND PAVEMENT AREAS TOWARDS APPROPRIATE DRAINAGE DEVICES TO AN OFF-SITE FACILITY. GRADES ADJACENT TO THE EXTERIO OF THE BUILDING SHALL BE SLOPED A MINIMUM OF 2% FOR A MINIMUM DISTANCE OF 5'-0" AWAY FROM THE BUILDING. ROOF DRAINS SHALL INSTALLED WITH APPROPRIATE DOWNSPOUT EXTENSION OUT-FALLING ON SPLASH BLOCKS AS TO DIRECT WATER A MINIMUM OF 5'-0" AWAY FROM THE BUILDING OR CONNECTED TO A STORM DRAIN SYSTEM.

GENERAL NOTES

TYPICAL SLAB ON GRADE:

5" THICK CONCRETE SLAB (F'c=3,000 psi, MAX W/C RATIO= 0.45) WITH #4 REBAR @ 16" o.c. E.W. INSTALLED AT CENTER OF SLAB OVER 2" SAND OVER VAPOR BARRIER (15 MIL, MIN) w/ SEALED JOINTS AND PENETRATION OVER 2" SAND OVER PRE-SOAKED COMPACTED SUB-BASE PER SOILS REPORT. SEE DETAIL 3 SD1

PROVIDE CONTROL JOINTS AT 15'-0" o.c. EA. WAY MAX.

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HCP ENGINEERING
2466 REDBROCK DR.
CORONA, CA 92881
P: 951.738.0940
F: 951.738.1432
www.hcpengineeringco.com

DESIGN ENGINEERING

HAMPTON INN - SANTA CLARITA
NEWHALL RANCH RD
Santa Clarita, CA 91355

PLAN CHECK NO: BLD19-00927

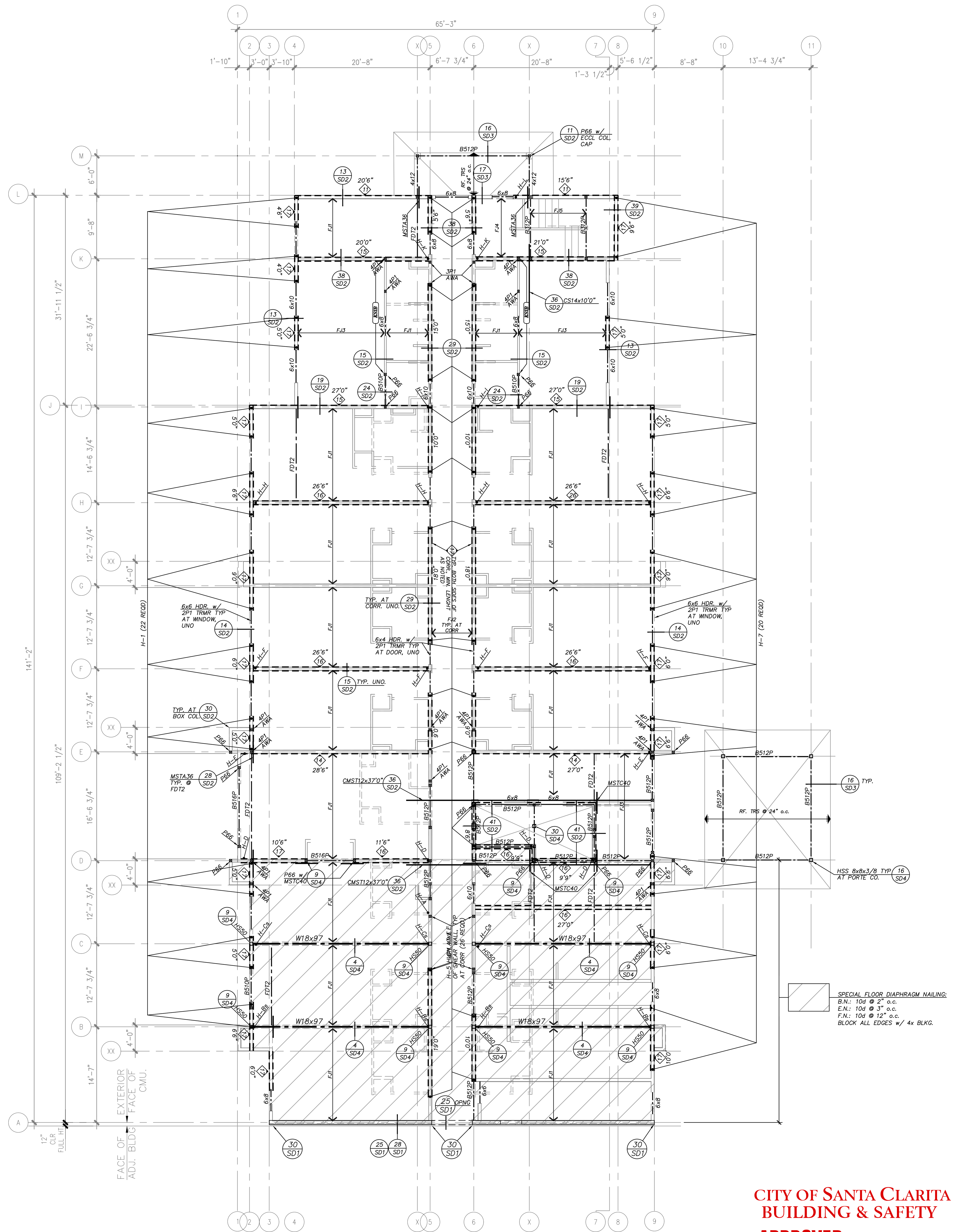
SEAL
REGISTERED PROFESSIONAL ENGINEER
HITESH C. PATEL
No. C50389
Exp. 6-30-2019
STATE OF CALIFORNIA
CIVIL
10/31/20

FOUNDATION PLAN

REVISION
9/12/19 1 PLAN CHECK #1
12/6/19 2 PLAN CHECK #2

PROJECT No.: 2018-14
PLOT DATE: October 31, 2020

SHEET
S1.0



1 2ND FLOOR FRAMING PLAN

SCALE 1/8"=1'-0"

BEAM / HEADER SCHEDULE

MARK	SIZE	TRIMMERS/POST,UNO
4x4	4x4	2x4
4x6	4x6 / 6x4	2x4
4x8	4x8	2P1 or 4x4
4x10	4x10	2P1 or 4x4
4x12	4x12	3P1 or 4x4
6x6	6x6	2P1 or 4x6
6x8	6x8	2P1 or 4x6
6x10	6x10	3P1 or 4x6
6x12	6x12	3P1 or 4x6
B212L	1 3/4" x 12" LSL.	2P1 or 4x4
B214L	1 3/4" x 14" LSL	2P1 or 4x4
B310P	3 1/2" x 9 1/2" PSL	3P1 or 4x4
B312P	3 1/2" x 11 7/8" PSL	4P1 or 4x6
B314P	3 1/2" x 14" PSL	4P1 or 4x6
B316P	3 1/2" x 16" PSL	4P1 or 4x6
B318P	3 1/2" x 18" PSL	4P1 or 4x6
B510P	5-1/4"x9 1/2" PSL	4P1 or 6x6
B512P	5-1/4"x11 7/8" PSL	6x6
B514P	5-1/4"x14" PSL	6x6
B516P	5-1/4"x16" PSL	6x6
B518P	5-1/4"x18" PSL	6x6
B520P	5-1/4"x20" PSL	6x6
B710P	7" x 9 1/2" PSL	8x4
B712P	7" x 11 7/8" PSL	8x6
B714P	7" x 14" PSL	8x6
B716P	7" x 16" PSL	8x6

BEAM/HEADER SCHEDULE NOTES:

1. TRIMMERS / POST SHALL BE INSTALLED AT EA. END OF BEAMS / HEADERS , U.N.O.

FLOOR JOIST SCHEDULE

MARK	SIZE
FJ-1	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 16'-6" (TYP. FLOOR LOAD)
FJ-2	2x8 @ 16" o.c. (Lmax= 6'-6") (CORR. LOAD)
FJ-3	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 14'-0" (FLOOR LOAD)
FJ-4	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 9'-8" (STAIR LOAD)
FJ-5	1 3/4"x14 LSL 1.55E STRINGERS @ 16" o.c. (STAIR LOAD), MAXIMUM SPAN LENGHT = 10'-0"

FLOOR JOIST NOTES:

FLOOR JOIST LOADS: (MIN.)

D.L.: TOP CHORD = 24.0 psf (SEE NOTE #1 & 2)
BOTT. CHORD = 8.0 psf (SEE NOTE #3)

L.L.: TOP CHORD = 40.0 psf TYP.
100.0 psf CORR.
125.0 psf STORAGE
60.0 psf BALCONY

NOTES:
1. PARTITION LOAD OF 10 psf (PARTITION HEIGHT) SHALL BE INCLUDED FOR NON-BEARING PARTITION AT PARTITION LOCATION.
2. ALL SUPERIMPOSED LOADS ABOVE JOIST SHALL BE INCLUDED IN JOIST DESIGN.
3. ADDITIONAL LOAD OF 250# DEAD LOAD ANY WHERE ALONG TRUSS BOTT. CHORD FOR FIRE SP/M/E/P EQUIP. ATTACHMENT.
4. DEFLECTION CRITERIA: TL = L/360, LL = L/480
5. DRAG TRUSSES (FDT#):
FDT2 = 2,000 lbs FDT6 = 6,000 lbs
FDT3 = 3,000 lbs FDT8 = 8,000 lbs
FDT4 = 4,000 lbs

WALL STUD FRAMING SCHEDULE

EXTERIOR WALLS	1st - 3x6 STUDS @ 16" o.c. DF#1 2nd - 2x6 STUDS @ 16" o.c. 3rd - 2x6 STUDS @ 16" o.c. 4th - 2x6 STUDS @ 16" o.c. 5th - 2x6 STUDS @ 16" o.c.	FIRE RATED STUDS, SEE NOTE #5
INTERIOR BEARING: (PARTY WALLS)	1st - 4x6 STUDS @ 16" o.c. 2nd - 2x6 STUDS @ 16" o.c. 3rd - 2x6 STUDS @ 16" o.c. 4th - 2x6 STUDS @ 16" o.c. 5th - 2x6 STUDS @ 16" o.c.	
INTERIOR BEARING: (CORR. AND ELEV. SHAFT WALLS)	1st - 3x6 STUDS @ 16" o.c., UNO. 2nd - 2x6 STUDS @ 16" o.c. 3rd - 2x6 STUDS @ 16" o.c. 4th - 2x6 STUDS @ 16" o.c. 5th - 2x6 STUDS @ 16" o.c.	
INTERIOR BEARING "KSIB": 6" WALLS (KING SUITE BATH ROOM) KSIB	1st - 4x6 STUDS @ 16" o.c., UNO. 2nd - 2x6 STUDS @ 16" o.c. 3rd - 2x6 STUDS @ 16" o.c. 4th - 2x6 STUDS @ 16" o.c. 5th - 2x6 STUDS @ 16" o.c.	
TOWER WALLS	2x4 STUD @ 16" o.c. - MAX HT.= 10"-0" 2x6 STUD @ 16" o.c. - MAX HT.= 15"-0"	
INTERIOR NON-BEARING WALLS	2x4 STUD @ 16" o.c. - MAX HT.= 14"-0" 2x6 STUD @ 16" o.c. - MAX HT.= 20"-0"	

WALL STUD SCHEDULE NOTES:

1. ALL STRUCTURAL STUD LUMBER SHALL BE DOUGLAS FIR LARCH #2 OR BETTER.
2. ALL SILL PLATES IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED.
3. SEE SHEARWALL SCHEDULE ON SN-1 FOR ADDITIONAL WALL FRAMING REQUIREMENTS
4. USE 3x PRESSURE TREATED FOUNDATION SILL PLATE AT SHEAR WALLS.
5. EXTERIOR WALL FRAMING SHALL BE FIRE RETARDANT TREATED WOOD AND SHEATHING. NAILS AT FIRE RETARDANT TREATED EOOD SHALL BE GALVANIZED.

ROOF TRUSS NOTES

TOP CHORD
D.L. = 26.5 PSF*
L.L. = 20.0 PSF

BOTTOM CHORD
D.L. = 5.5 PSF (10.0 PSF @ EXT.)
** L.L. = 10.0 PSF

* D.L. INCLUDES 12.0 PSF ALLOWANCE FOR FUTURE SOLAR SYS.

WIND
WIND SPEED = 110MPH
EXPOSURE = C

** NOT CONCURRENT w/ TOP CHORD LIVE LOAD.

BRNG WALL / BEAM — TRS. SPAN — TRS. SPAN — CANT'L

TRUSS NOTES:

1. TOP CHORD SLOPE SHALL BE AS SPECIFIED BY ARCHITECT
2. EACH JOIST SHALL BE LEGIBLE MARKED ON THE BOTTOM CHORD WITH COMPANY NAME AND TRUSS IDENTIFICATION NUMBER.
3. A CERTIFICATION OF COMPLIANCE IS REQUIRED FOR EVERY TRUSS.
4. SEE ROOF PLAN FOR LOCATION AND WEIGHT OF MECH. EQUIPMENT SUPPORTED ON THE ROOF..
5. PROVIDE FULL DEPTH JOIST BLOCKING AT TRUSSES PERPENDICULAR TO SHEAR WALLS. BLOCKING SHALL EXTEND ENTIRE LENGTH OF SHEAR WALL.
6. PROVIDE BLOCKING AT PARAPET WALL BRACE ATTACHMENT / SUPPORT.
7. TRUSS BOTTOM CHORD SHALL BE DESIGNED FOR 250# DEAD LOAD SP/M/P/E EQUIP. ATTACHMENT ALONG ANYWHERE ALONG THE BOTTOM CHORD.
8. SEE SPECIAL GIRDER TRUSS LOAD TABLE (SHEET S-6.0) FOR ADDITIONAL LOADS AT TRUSSES SUPPORTING TOWER WALLS.
9. G.E.T = GABLE END TRUSS, SEE PLANS FOR DRAG LOAD.
10. BOUNDRY NAIL (B.N.) ROOF SHEATHING TO DRAG TRUSS, UNO.
11. 'RDT#' INDICATES DRAG TRUSS. DRAG TRUSS SHALL BE DESIGNED/SIZED FOR STRAP ATTACHMENT FOR STRAP SIZES SHOWN ON PLANS. TRUSS SHALL BE DESIGN FOR THE FOLLOWING MINIMUM DRAG CAPACITY:

DTW = PLF SAME AT SHEAR WALL BELOW
RDT2 = 2,000 lbs

GENERAL NOTES

CONTRACTOR TO CONFIRM BEAM SIZES, LOCATIONS, CALL OUTS, AND SHEAR WALL LOCATIONS. BRING TO THE ARCHITECTS ATTENTION ANY DISCREPANCIES FOUND PRIOR TO START OF CONSTRUCTION.

CONTRACTOR TO REVIEW W/ FRAMING CONTRACTOR THE FLOOR PLANS AND INTERIOR ELEVATION SHEET AND VERIFY ALL SOFFITS, ARCHES AND SPECIAL FRAMING CONDITIONS AND REPORT TO THE ARCHITECT ANY DISCREPANCIES PRIOR TO THE START OF CONSTRUCTION.

CONTRACTOR TO SCHEDULE FRAMING WALK-THROUGH WITH ENGINEER PRIOR TO INSULATION AND DRYWALL. ENGINEER TO GIVE CONTRACTOR WRITTEN RESPONSE OF WALK-THROUGH IN A FRAMING CERTIFICATION LETTER.

FLOOR/ROOF FRAMING NOTES

1. SEE GENERAL NOTES ON SHEET SN-1 AND SN-2
2. SEE STRUCTURAL STUD WALL SCHEDULE ON THIS DRAWING. SEE ARCHITECTURAL DRAWINGS FOR NON-LOAD BEARING PARTITIONS.
3. ALL BEARING STUD WALLS SHALL BE BLOCKED AT MID HEIGHT AS A MINIMUM. SEE SHEAR WALL SCHEDULE FOR PLYWOOD EDGE NAILING, SEE SHEET SN-1.
4. MARKS "◇" INDICATES PLYWOOD SHEAR WALLS WITH SHEATHING ON SIDE(S) OF WALL NOTED. SEE SHEAR WALL SCHEDULE ON SHEET SN-1.

MARKS "◇" INDICATES PLYWOOD SHEAR WALLS WITH SHEATHING ON BOTH SIDES OF WALL.

5. FOR ROOF DRAINS, OPENINGS, FASCIA DETAILS, ETC., SEE DRAWINGS OTHER THAN STRUCTURAL.
6. FRAME WALL OPENINGS PER DETAIL 1/SD2
7. ALL NON-BEARING INTERIOR WALLS PER DETAIL 3/SD2
8. TOP PLATE SPLICE PER DETAIL 2/SD2
9. -
10. TYPICAL FLOOR SHEATHING:
3/4" T&G UNDERLAYMENT GRADE PLYWOOD(P.I. 48/24),
B.N. PLY: 10d @ 6" o.c.
E.N. PLY: 10d @ 6" o.c.
F.N. PLY: 10d @ 12" o.c.
GLUE ALL CONTACT SURFACES.
11. TYPICAL ROOF SHEATHING:
5/8" CDX PLYWOOD(P.I. 32/16)
B.N. PLY: 8d @ 6" o.c.
E.N. PLY: 8d @ 6" o.c.
F.N. PLY: 8d @ 12" o.c.
PROVIDE PANEL EDGE CLIP MIDWAY BETWEEN EACH SUPPORT OR PROVIDE T&G PLYWOOD.
12. ▨ INDICATES SPECIAL ROOF/FLOOR DIAPHRAGM WITH ALL EDGES BLOCKED. SEE PLANS FOR SPECIAL NAILING REQUIREMENTS.
13.
14.
15. PROVIDE BLOCKING UNDER WALL PARALLEL TO JOIST PER DETAIL 17/SD2 TYP. U.N.O.
16. PROVIDE SOLID BLOCKING UNDER PERPENDICULAR WALLS PER DETAIL 17/SD2 TYP. U.N.O.
17. NON-SHEAR WALLS ADJACENT TO AND PARALLEL WITH SHEAR WALLS SHALL RECEIVE PLYWOOD SHEATHING TO PROVIDE SMOOTH TRANSITION FROM SHEAR WALLS TO NON-SHEAR WALLS.
18. REFER TO ARCHITECTURAL DRAWINGS FOR NON-STRUCTURAL SOFFIT FRAMING.
19. ABBREVIATIONS:

EFW = ENTIRE FACE OF WALL
G.T. = GIRDER TRUSS
J.T. = JACK TRUSS
D.T. = DRAG TRUSS w/ PLY SHTH'G B.N. (DRAG LOAD AS SPECIFIED)
FL = FLUSH BEAM
DP = DROP BEAM
M.S. = MULITIPLE STUDS (WIDTH TO MATCH BEAM, 2-2xSTUDS MIN.)
E.E. = SAME CONDITION OCCURS AT EACH END OF BEAM
D.G = DRAG JOIST/BEAM w/ PLY. SHT'G B.N.
BS = CONDITION OCCUR ON BOTH SIDES OF WALL, BEAM...etc
UNO = UNLESS NOTED OTHERWISE
AWA = ALIGN WITH ABOVE
PA = POST ABOVE
GET = GABLE END TRUSS (DRAG TRUSS WHERE NOTED)
TRM = TRIMMER STUS / POSTS AT HEADERS
KP = KING POST
C.P. = CRIPPLE POST
⊠ = MULTIPLE STUD OR POST (2-2x STUDS, MIN. UNO.)
** = SPECIAL ALIGNMENT OF POST/HOLDOWN w/ CONDITION ABOVE OR BELOW.

20. PLUMBING, ELECTRICAL, MECHANICAL AND FIRESPRINKLER SYSTEMS SHALL BE DESIGNED AND INSTALLED TO ACCOMODATE WOOD SHRINKAGE (SETTLEMENT) AS FOLLOWS:
1st FLOOR: 1/4" TOTAL VERTICAL MOVEMENT
2nd FLOOR: 3/8" "
3rd FLOOR: 1/2" "
4th FLOOR: 5/8" "

POST ABREVATION

MARK	SIZE	MARK	SIZE
#P1	SEE NOTE #1	HS50	HSS 5x5x5/16
P44	4x4 POST	-	-
P46	4x6 POST	-	-
P48	4x8 POST	-	-
P66	6x6 POST	-	-
P68	6x8 POST	-	-

POST NOTES:
1. USE 2-2x STUDS MIN. (# = TOTAL NUMBER OF STUDS WHERE MORE THAN 2-2x STUDS ARE REQUIRED, Ex: 3P1 = 3-2x STUDS), STITCH NAIL EA. STUD w/ 16d @ 12" o.c.

HOLDOWN / VERT. STRAP SCHED.

MARK	SIZE	DETAIL
V40 V52 V66 V78 V146 V128 V168	MSTC40 (v) MSTC52 (v) MSTC66 (v) MSTC78 (v) CS14 x 6'-0" (v) CMST12 x 8'-0" (v) CMSTC16 x 8'-0" (v)	32/SD2
U8 U11 U14 D19	HDU8 HLDN HDU11 HLDN HDU14 HLDN HD19	32/SD2
H-#	INDICATES SPECIAL PATENTED HOLDDOWN SYSTEM. COORDINATE LOCATION WITH FRAMING AT LEVEL ABOVE (** INDICATES SPECIAL ALIGNMENT REQUIRED WITH ABOVE OR BELOW)	SEE SHEETS ATS-1 & ATS-2

LEGEND:

#/SHT — DETAIL NUMBER / SHEET REFERENCE
SHEAR WALL TYPE
X'X" — SHEAR WALL LENGTH
XX — INDICATES FLOOR JOIST. SEE FLOOR JOIST SCHEDULE, U.N.O.
XX — INDICATES ROOF TRUSSES @ 24" o.c.
V-##, U-##, H-## — INDICATES HOLDOWN / VERTICAL STRAP, SEE HOLDOWN SCHEDULE FOR ADDITIONAL INFORMATION.
⊠ POST OR TRIMMER STUDS AT BEAMS. USE 2-2x MIN. UNO.
☐ STEEL COLUMN
WALL BELOW CURRENT FRAMING LEVEL
WALL ABOVE CURRENT FRAMING LEVEL

HCP ENGINEERING
2466 REDROCK DR.
CORONA, CA 92881
P: 951.738.0840
F: 951.738.1432
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HCP ENGINEERING DESIGN

HAMPTON INN - SANTA CLARITA

NEWHALL RANCH RD
Santa Clarita, CA 91355

PLAN CHECK NO: BLD19-00927

SEAL

REGISTERED PROFESSIONAL ENGINEER
HITESH C. PATEL
No. C50389
Exp. 6-30-2019
CIVIL
STATE OF CALIFORNIA

[illegible]

10/31/20

2ND FLOOR FRAMING PLAN

REVISION

9/12/19	1	PLAN CHECK #1
12/6/19	2	PLAN CHECK #2

PROJECT No.: 2018-14

PLOT DATE: October 31, 2020

SHEET: S-2.0

CITY OF SANTA CLARITA
BUILDING & SAFETY
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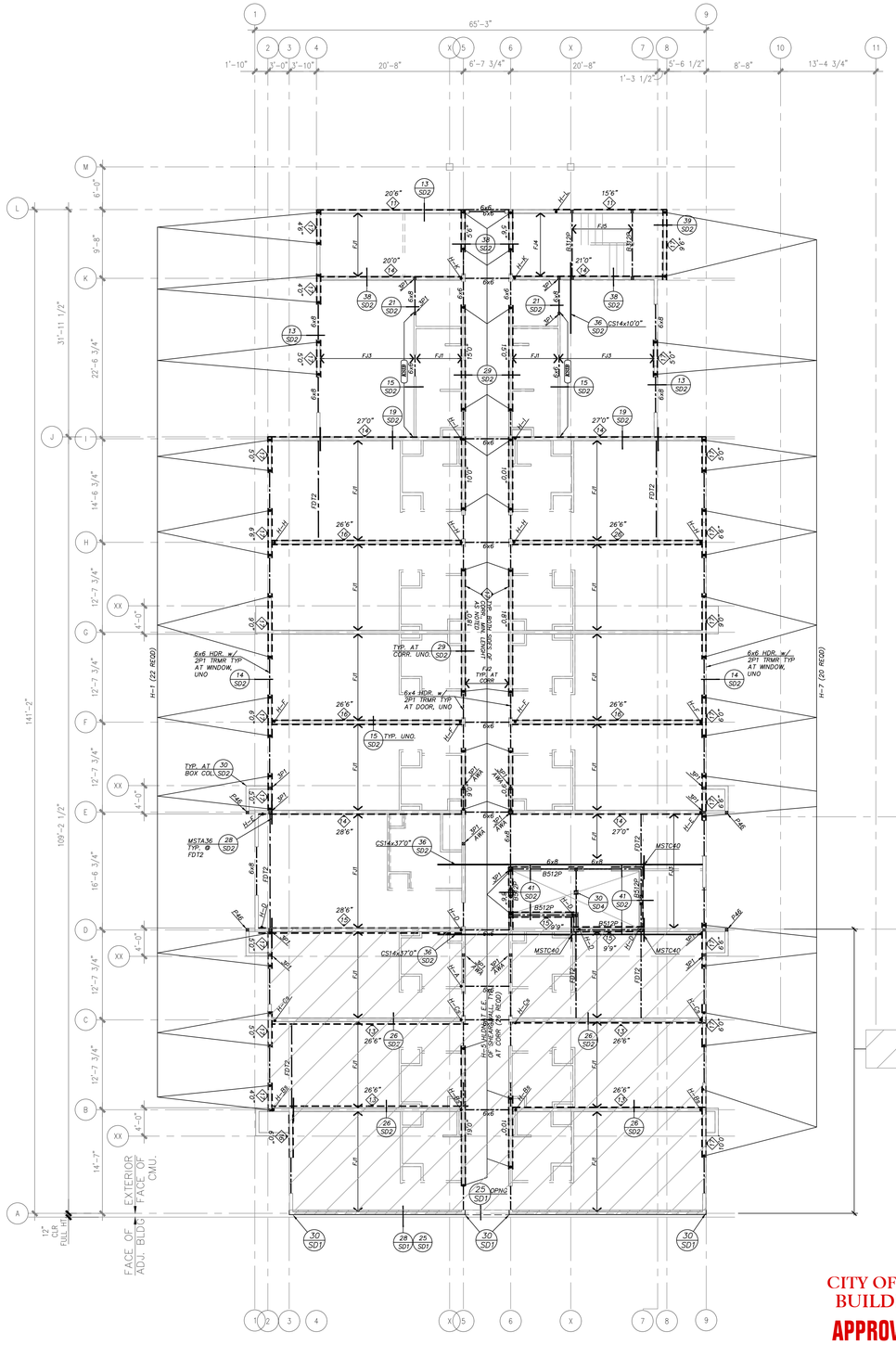

1 3RD FLOOR FRAMING PLAN
SCALE 1/8"=1'-0"

SPECIAL FLOOR DIAPHRAGM NAILING:
B.N.: 10d @ 6" o.c.
E.N.: 10d @ 6" o.c.
F.N.: 10d @ 12" o.c.
BLOCK ALL EDGES

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BEAM / HEADER SCHEDULE

MARK	SIZE	TRIMMERS/POST,UNO
4x4	4x4	2x4
4x6	4x6 / 6x4	2x4
4x8	4x8	2P1 or 4x4
4x10	4x10	2P1 or 4x4
4x12	4x12	3P1 or 4x4
6x6	6x6	2P1 or 4x6
6x8	6x8	2P1 or 4x6
6x10	6x10	3P1 or 4x6
6x12	6x12	3P1 or 4x6
B212L	1 3/4" x 12" LSL	2P1 or 4x4
B214L	1 3/4" x 14" LSL	2P1 or 4x4
B310P	3 1/2" x 9 1/2" PSL	3P1 or 4x4
B312P	3 1/2" x 11 7/8" PSL	4P1 or 4x6
B314P	3 1/2" x 14" PSL	4P1 or 4x6
B316P	3 1/2" x 16" PSL	4P1 or 4x6
B318P	3 1/2" x 18" PSL	4P1 or 4x6
B510P	5-1/4"x9 1/2" PSL	4P1 or 6x6
B512P	5-1/4"x11 7/8" PSL	6x6
B514P	5-1/4"x14" PSL	6x6
B516P	5-1/4"x16" PSL	6x6
B518P	5-1/4"x18" PSL	6x6
B520P	5-1/4"x20" PSL	6x6
B710P	7" x 9 1/2" PSL	8x4
B712P	7" x 11 7/8" PSL	8x6
B714P	7" x 14" PSL	8x6
B716P	7" x 16" PSL	8x6

BEAM/HEADER SCHEDULE NOTES:

1. TRIMMERS / POST SHALL BE INSTALLED AT EA. END OF BEAMS / HEADERS , U.N.O.

FLOOR JOIST SCHEDULE

MARK	SIZE
FJ-1	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 16'-6" (TYP. FLOOR LOAD)
FJ-2	2x8 @ 16" o.c. (Lmax= 6'-6") (CORR. LOAD)
FJ-3	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 14'-0" (TYP. FLOOR LOAD)
FJ-4	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 9'-8" (STAIR LOAD)
FJ-5	1 3/4"x14 LSL 1.55E STRINGERS @ 16" o.c. (STAIR LOAD), MAXIMUM SPAN LENGHT = 10'-0"

FLOOR JOIST NOTES:

FLOOR JOIST LOADS: (MIN.)
D.L.: TOP CHORD = 24.0 psf (SEE NOTE #1 & 2)
BOTT. CHORD = 8.0 psf (SEE NOTE #3)
L.L.: TOP CHORD = 40.0 psf TYP.
100.0 psf CORR.
125.0 psf STORAGE
60.0 psf BALCONY

NOTES:
1. PARTITION LOAD OF 10 psf (PARTITION HEIGHT) SHALL BE INCLUDED FOR NON-BEARING PARTITION AT PARTITION LOCATION.
2. ALL SUPERIMPOSED LOADS ABOVE JOIST SHALL BE INCLUDED IN JOIST DESIGN.
3. ADDITIONAL LOAD OF 250# DEAD LOAD ANY WHERE ALONG TRUSS BOTT. CHORD FOR FIRE SP/M/E/P EQUIP. ATTACHMENT.
4. DEFLECTION CRITERIA: TL = L/360, LL = L/480
5. DRAG TRUSSES (FDT#):
FDT2 = 2,000 lbs FDT6 = 6,000 lbs
FDT3 = 3,000 lbs FDT8 = 8,000 lbs
FDT4 = 4,000 lbs

WALL STUD FRAMING SCHEDULE

EXTERIOR WALLS	1st – 3x6 STUDS @ 16" o.c. DF#1 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	FIRE RATED STUDS, SEE NOTE #5
INTERIOR BEARING: (PARTY WALLS)	1st – 4x6 STUDS @ 16" o.c. 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	
INTERIOR BEARING: (CORR. AND ELEV. SHAFT WALLS)	1st – 3x6 STUDS @ 16" o.c., UNO. 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	
INTERIOR BEARING "XSIB" 6" WALLS (KING SUITE BATH ROOM) (7"SIB)	1st – 4x6 STUDS @ 16" o.c., UNO. 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	
TOWER WALLS	2x4 STUD @ 16" o.c. – MAX HT.= 10'-0" 2x6 STUD @ 16" o.c. – MAX HT.= 15'-0"	
INTERIOR NON-BEARING WALLS	2x4 STUD @ 16" o.c. – MAX HT.= 14'-0" 2x6 STUD @ 16" o.c. – MAX HT.= 20'-0"	

WALL STUD SCHEDULE NOTES:

1. ALL STRUCTURAL STUD LUMBER SHALL BE DOUGLAS FIR LARCH #2 OR BETTER.
2. ALL SILL PLATES IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED.
3. SEE SHEARWALL SCHEDULE ON SN-1 FOR ADDITIONAL WALL FRAMING REQUIREMENTS
4. USE 3x PRESSURE TREATED FOUNDATION SILL PLATE AT SHEAR WALLS.
5. EXTERIOR WALL FRAMING SHALL BE FIRE RETARDANT TREATED WOOD AND SHEATHING. NAILS AT FIRE RETARDANT TREATED EOOD SHALL BE GALVANIZED.

ROOF TRUSS NOTES

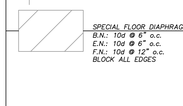
TOP CHORD
D.L. = 26.5 PSF*
L.L. = 20.0 PSF

BOTTOM CHORD
D.L. = 5.5 PSF (10.0 PSF @ EXT.)
** L.L. = 10.0 PSF

* D.L. INCLUDES 12.0 PSF ALLOWANCE FOR FUTURE SOLAR SYS.

WIND
WIND SPEED = 110MPH
EXPOSURE = C

** NOT CONCURRENT w/ TOP CHORD LIVE LOAD.



TRUSS NOTES:

1. TOP CHORD SLOPE SHALL BE AS SPECIFIED BY ARCHITECT
2. EACH JOIST SHALL BE LEGIBLE MARKED ON THE BOTTOM CHORD WITH COMPANY NAME AND TRUSS IDENTIFICATION NUMBER.
3. A CERTIFICATION OF COMPLIANCE IS REQUIRED FOR EVERY TRUSS.
4. SEE ROOF PLAN FOR LOCATION AND WEIGHT OF MECH. EQUIPMENT SUPPORTED ON THE ROOF.
5. PROVIDE FULL DEPTH JOIST BLOCKING AT TRUSSES PERPENDICULAR TO SHEAR WALLS. BLOCKING SHALL EXTEND ENTIRE LENGTH OF SHEAR WALL.
6. PROVIDE BLOCKING AT PARAPET WALL BRACE ATTACHMENT / SUPPORT.
7. TRUSS BOTTOM CHORD SHALL BE DESIGNED FOR 250# DEAD LOAD SP/M/P/E EQUIP. ATTACHMENT ALONG ANYWHERE ALONG THE BOTTOM CHORD.
8. SEE SPECIAL GIRDER TRUSS LOAD TABLE (SHEET S-6.0) FOR ADDITIONAL LOADS AT TRUSSES SUPPORTTING TOWER WALLS.
9. G.E.T = GABLE END TRUSS, SEE PLANS FOR DRAG LOAD.
10. BOUNDRY NAIL (B.N.) ROOF SHEATHING TO DRAG TRUSS, UNO.
11. "ROT#" INDICATES DRAG TRUSS. DRAG TRUSS SHALL BE DESIGNED/SIZED FOR STRAP ATTACHMENT FOR STRAP SIZES SHOWN ON PLANS. TRUSS SHALL BE DESIGN FOR THE FOLLOWING MINIMUM DRAG CAPACITY:
DTW = PLF SAME AT SHEAR WALL BELOW
RDT2 = 2,000 lbs

GENERAL NOTES

CONTRACTOR TO CONFIRM BEAM SIZES, LOCATIONS, CALL OUTS, AND SHEAR WALL LOCATIONS. BRING TO THE ARCHITECTS ATTENTION ANY DISCREPANCIES FOUND PRIOR TO START OF CONSTRUCTION.

CONTRACTOR TO REVIEW W/ FRAMING CONTRACTOR THE FLOOR PLANS AND INTERIOR ELEVATION SHEET AND VERIFY ALL SOFFITS, ARCHES AND SPECIAL FRAMING CONDITIONS AND REPORT TO THE ARCHITECT ANY DISCREPANCIES PRIOR TO THE START OF CONSTRUCTION.

CONTRACTOR TO SCHEDULE FRAMING WALK-THROUGH WITH ENGINEER PRIOR TO INSULATION AND DRYWALL. ENGINEER TO GIVE CONTRACTOR WRITTEN RESPONSE OF WALK-THROUGH IN A FRAMING CERTIFICATION LETTER.

FLOOR/ROOF FRAMING NOTES

1. SEE GENERAL NOTES ON SHEET SN-1 AND SN-2
2. SEE STRUCTURAL STUD WALL SCHEDULE ON THIS DRAWING. SEE ARCHITECTURAL DRAWINGS FOR NON-LOAD BEARING PARTITIONS.
3. ALL BEARING STUD WALLS SHALL BE BLOCKED AT MID HEIGHT AS A MINIMUM. SEE SHEAR WALL SCHEDULE FOR PLYWOOD EDGE NAILING, SEE SHEET SN-1.
4. MARKS "◇" INDICATES PLYWOOD SHEAR WALLS WITH SHEATHING ON SIDE(S) OF WALL NOTED. SEE SHEAR WALL SCHEDULE ON SHEET SN-1.
MARKS "◇" INDICATES PLYWOOD SHEAR WALLS WITH SHEATHING ON BOTH SIDES OF WALL.
5. FOR ROOF DRAINS, OPENINGS, FASCIA DETAILS, ETC., SEE DRAWINGS OTHER THAN STRUCTURAL.
6. FRAME WALL OPENINGS PER DETAIL 1/SD2
7. ALL NON-BEARING INTERIOR WALLS PER DETAIL 3/SD2
8. TOP PLATE SPLICE PER DETAIL 2/SD2
9. –
10. TYPICAL FLOOR SHEATHING:
3/4" T&G UNDERLAYMENT GRADE PLYWOOD(P.I. 48/24),
B.N. PLY: 10d @ 6" o.c.
E.N. PLY: 10d @ 6" o.c.
F.N. PLY: 10d @ 12" o.c.
GLUE ALL CONTACT SURFACES.
11. TYPICAL ROOF SHEATHING:
5/8" CDX PLYWOOD(P.I. 32/16)
B.N. PLY: 8d @ 6" o.c.
E.N. PLY: 8d @ 6" o.c.
F.N. PLY: 8d @ 12" o.c.
PROVIDE PANEL EDGE CLIP MIDWAY BETWEEN EACH SUPPORT OR PROVIDE T&G PLYWOOD.
12. INDICATES SPECIAL ROOF/FLOOR DIAPHRAGM WITH ALL EDGES BLOCKED. SEE PLANS FOR SPECIAL NAILING REQUIREMENTS.
13. –
14. –
15. PROVIDE BLOCKING UNDER WALL PARALLEL TO JOIST PER DETAIL 17/SD2 TYP. U.N.O.
16. PROVIDE SOLID BLOCKING UNDER PERPENDICULAR WALLS PER DETAIL 17/SD2 TYP. U.N.O.
17. NON-SHEAR WALLS ADJACENT TO AND PARALLEL WITH SHEAR WALLS SHALL RECEIVE PLYWOOD SHEATHING TO PROVIDE SMOOTH TRANSITION FROM SHEAR WALLS TO NON-SHEAR WALLS.
18. REFER TO ARCHITECTURAL DRAWINGS FOR NON-STRUCTURAL SOFFIT FRAMING.
19. ABBREVIATIONS:

EFW =	ENTIRE FACE OF WALL
G.T. =	GIRDER TRUSS
J.T. =	JACK TRUSS
D.T. =	DRAG TRUSS w/ PLY SHTH'G B.N. (DRAG LOAD AS SPECIFIED)
FL =	FLUSH BEAM
DP =	DROP BEAM
M.S. =	MULTIPLE STUDS (WIDTH TO MATCH BEAM, 2-2xSTUDS MIN.)
E.E. =	SAME CONDITION OCCURS AT EACH END OF BEAM
D.G =	DRAG JOIST/BEAM w/ PLY. SHT'G B.N.
BS =	CONDITION OCCUR ON BOTH SIDES OF WALL, BEAM...etc
UNO =	UNLESS NOTED OTHERWISE
AWA =	ALIGN WITH ABOVE
PA =	POST ABOVE
GET =	GABLE END TRUSS (DRAG TRUSS WHERE NOTED)
TRM =	TRIMMER STUS / POSTS AT HEADERS
KP =	KING POST
C.P. =	CRIPPLE POST
⊠ =	MULTIPLE STUD OR POST (2-2x STUDS, MIN. UNO.)
** =	SPECIAL ALIGNMENT OF POST/HOLDOWN w/ CONDITION ABOVE OR BELOW.

20. PLUMBING, ELECTRICAL, MECHANICAL AND FIRESPRINKLER SYSTEMS SHALL BE DESIGNED AND INSTALLED TO ACCOMODATE WOOD SHRINKAGE (SETTLEMENT) AS FOLLOWS:
1st FLOOR: 1/4" TOTAL VERTICAL MOVEMENT
2nd FLOOR: 3/8" "
3rd FLOOR: 1/2" "
4th FLOOR: 5/8" "

POST ABREVATION

MARK	SIZE	MARK	SIZE
#P1	SEE NOTE #1	HS50	HSS 5x5x5/16
P44	4x4 POST	–	–
P46	4x6 POST	–	–
P48	4x8 POST	–	–
P86	6x6 POST	–	–
P88	6x8 POST	–	–

POST NOTES:
1. USE 2-2x STUDS MIN. (# = TOTAL NUMBER OF STUDS WHERE MORE THAN 2-2x STUDS ARE REQUIRED, Ex: 3P1 = 3-2x STUDS), STITCH NAIL EA. STUD w/ 16d @ 12" o.c.

HOLDOWN / VERT. STRAP SCHED.

MARK	SIZE	DETAIL
V40	MSTC40 (v)	32/SD2
V52	MSTC52 (v)	
V66	MSTC66 (v)	
V78	MSTC78 (v)	
V146	CS14 x 6'-0" (v)	
V128	CMST12 x 8'-0" (v)	
V168	CMSTC16 x 8'-0" (v)	
U8	HDU8 HLDN	32/SD2
U11	HDU11 HLDN	
U14	HDU14 HLDN	
D19	HD19	
H-#	INDICATES SPECIAL PATENTED HOLDDOWN SYSTEM. COORDINATE LOCATION WITH FRAMING AT LEVEL ABOVE (** INDICATES SPECIAL ALIGNMENT REQUIRED WITH ABOVE OR BELOW)	SEE SHEETS ATS-1 & ATS-2

LEGEND:

- DETAIL NUMBER / SHEET REFERENCE
- SHEAR WALL TYPE / SHEAR WALL LENGTH
- XX — INDICATES FLOOR JOIST. SEE FLOOR JOIST SCHEDULE, U.N.O.
- XX — INDICATES ROOF TRUSSES @ 24" o.c.
- V-## / U-## / H-## — INDICATES HOLDOWN / VERTICAL STRAP. SEE HOLDOWN SCHEDULE FOR ADDITIONAL INFORMATION.
- ⊠ POST OR TRIMMER STUDS AT BEAMS. USE 2-2x MIN. UNO.
- □ STEEL COLUMN
- WALL BELOW CURRENT FRAMING LEVEL
- WALL ABOVE CURRENT FRAMING LEVEL

HCP ENGINEERING
2466 REDBROOK DR.
CORONA, CA 92881
P: 951.738.0840
F: 951.738.1432
www.hcpengineeringco.com

DESIGN ENGINEERING

HAMPTON INN - SANTA CLARITA
NEWHALL RANCH RD
Santa Clarita, CA 91355

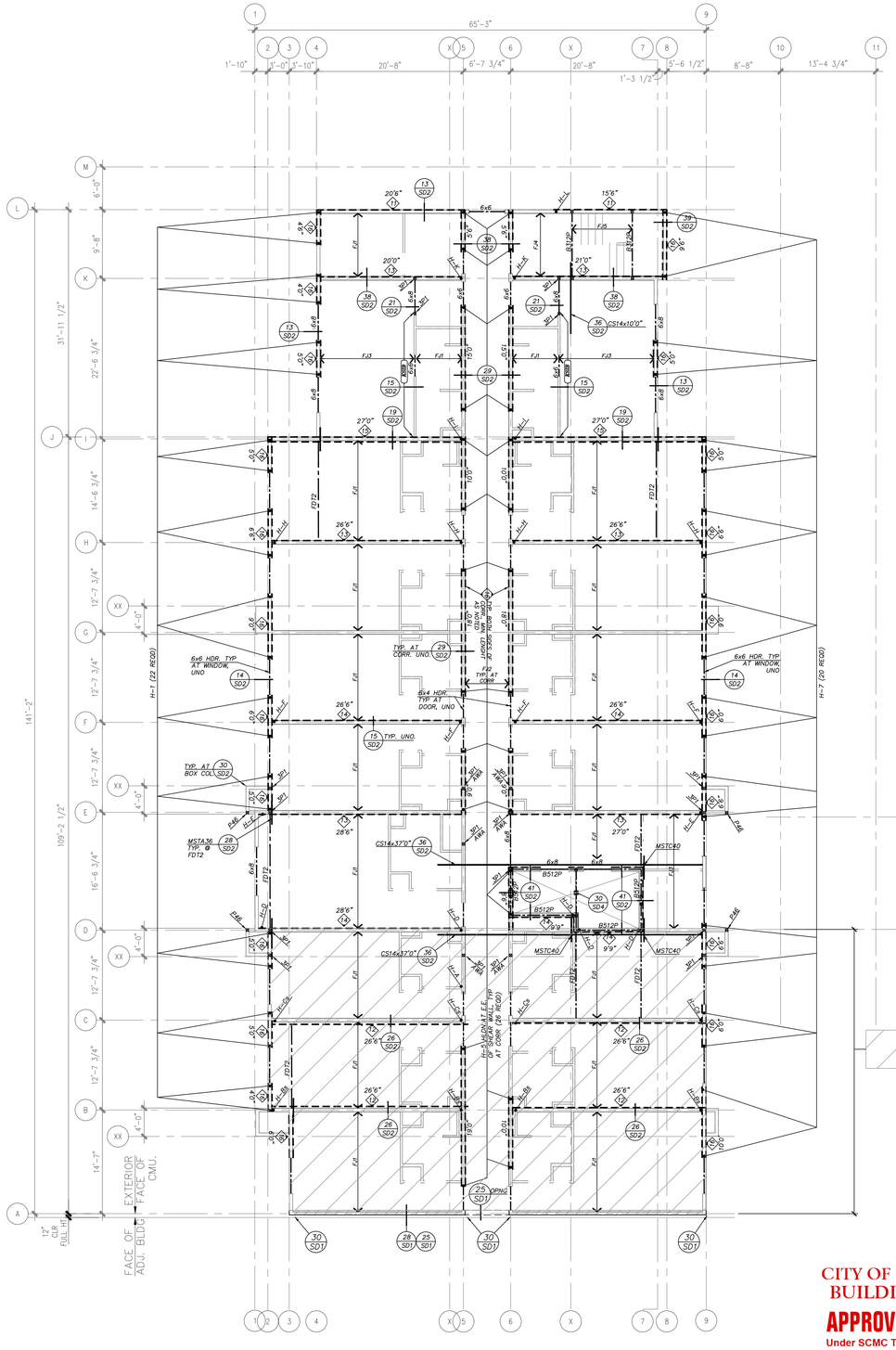
PLAN CHECK NO: BLD19-00927

SEAL
REGISTERED PROFESSIONAL ENGINEER
HITESH C. PATEL
No. C50389
Exp. 6-30-2019
STATE OF CALIFORNIA
CIVIL
10/31/20

3RD FLOOR FRAMING PLAN

REVISION
9/12/19 1 PLAN CHECK #1
12/6/19 2 PLAN CHECK #2

PROJECT No.: 2018-14
PLOT DATE: October 31, 2020
SHEET: **S-3.0**



1 4TH FLOOR FRAMING PLAN
SCALE 1/8"=1'-0"

BEAM / HEADER SCHEDULE

MARK	SIZE	TRIMMERS/POST,UNO
4x4	4x4	2x4
4x6	4x6 / 6x4	2x4
4x8	4x8	2P1 or 4x4
4x10	4x10	2P1 or 4x4
4x12	4x12	3P1 or 4x4
6x6	6x6	2P1 or 4x6
6x8	6x8	2P1 or 4x6
6x10	6x10	3P1 or 4x6
6x12	6x12	3P1 or 4x6
B212L	1 3/4" x 12" LSL	2P1 or 4x4
B214L	1 3/4" x 14" LSL	2P1 or 4x4
B310P	3 1/2" x 9 1/2" PSL	3P1 or 4x4
B312P	3 1/2" x 11 7/8" PSL	4P1 or 4x6
B314P	3 1/2" x 14" PSL	4P1 or 4x6
B316P	3 1/2" x 16" PSL	4P1 or 4x6
B318P	3 1/2" x 18" PSL	4P1 or 4x6
B510P	5-1/4"x9 1/2" PSL	4P1 or 6x6
B512P	5-1/4"x11 7/8" PSL	6x6
B514P	5-1/4"x14" PSL	6x6
B516P	5-1/4"x16" PSL	6x6
B518P	5-1/4"x18" PSL	6x6
B520P	5-1/4"x20" PSL	6x6
B710P	7" x 9 1/2" PSL	8x4
B712P	7" x 11 7/8" PSL	8x6
B714P	7" x 14" PSL	8x6
B716P	7" x 16" PSL	8x6

BEAM/HEADER SCHEDULE NOTES:

1. TRIMMERS / POST SHALL BE INSTALLED AT EA. END OF BEAMS / HEADERS , U.N.O.

FLOOR JOIST SCHEDULE

MARK	SIZE
FJ-1	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 16'-6" (TYP. FLOOR LOAD)
FJ-2	2x8 @ 16" o.c. (Lmax= 6'-6") (CORR. LOAD)
FJ-3	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 14'-0" (TYP. FLOOR LOAD)
FJ-4	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 9'-8" (STAIR LOAD)
FJ-5	1 3/4"x14 LSL 1.55E STRINGERS @ 16" o.c. (STAIR LOAD), MAXIMUM SPAN LENGHT = 10'-0"

FLOOR JOIST NOTES:

FLOOR JOIST LOADS: (MIN.)

D.L.: TOP CHORD = 24.0 psf (SEE NOTE #1 & 2)
BOTT. CHORD = 8.0 psf (SEE NOTE #3)

L.L.: TOP CHORD = 40.0 psf TYP.
100.0 psf CORR.
125.0 psf STORAGE
60.0 psf BALCONY

NOTES:
1. PARTITION LOAD OF 10 psf (PARTITION HEIGHT) SHALL BE INCLUDED FOR NON-BEARING PARTITION AT PARTITION LOCATION.
2. ALL SUPERIMPOSED LOADS ABOVE JOIST SHALL BE INCLUDED IN JOIST DESIGN.
3. ADDITIONAL LOAD OF 250# DEAD LOAD ANY WHERE ALONG TRUSS BOTT. CHORD FOR FIRE SP/M/E/P EQUIP. ATTACHMENT.
4. DEFLECTION CRITERIA: TL = L/360, LL = L/480
5. DRAG TRUSSES (FDT#):
FDT2 = 2,000 lbs FDT6 = 6,000 lbs
FDT3 = 3,000 lbs FDT8 = 8,000 lbs
FDT4 = 4,000 lbs

WALL STUD FRAMING SCHEDULE

EXTERIOR WALLS	1st – 3x6 STUDS @ 16" o.c. DF#1 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	FIRE RATED STUDS, SEE NOTE #5
INTERIOR BEARING: (PARTY WALLS)	1st – 4x6 STUDS @ 16" o.c. 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	
INTERIOR BEARING: (CORR. AND ELEV. SHAFT WALLS)	1st – 3x6 STUDS @ 16" o.c., UNO. 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	
INTERIOR BEARING "XSIB" 6" WALLS (KING SUITE BATH ROOM) (7"SIB)	1st – 4x6 STUDS @ 16" o.c., UNO. 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	
TOWER WALLS	2x4 STUD @ 16" o.c. – MAX HT.= 10'-0" 2x6 STUD @ 16" o.c. – MAX HT.= 15'-0"	
INTERIOR NON-BEARING WALLS	2x4 STUD @ 16" o.c. – MAX HT.= 14'-0" 2x6 STUD @ 16" o.c. – MAX HT.= 20'-0"	

WALL STUD SCHEDULE NOTES:

1. ALL STRUCTURAL STUD LUMBER SHALL BE DOUGLAS FIR LARCH #2 OR BETTER.
2. ALL SILL PLATES IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED.
3. SEE SHEARWALL SCHEDULE ON SN-1 FOR ADDITIONAL WALL FRAMING REQUIREMENTS
4. USE 3x PRESSURE TREATED FOUNDATION SILL PLATE AT SHEAR WALLS.
5. EXTERIOR WALL FRAMING SHALL BE FIRE RETARDANT TREATED WOOD AND SHEATHING. NAILS AT FIRE RETARDANT TREATED EOOD SHALL BE GALVANIZED.

ROOF TRUSS NOTES

TOP CHORD
D.L. = 26.5 PSF*
L.L. = 20.0 PSF

BOTTOM CHORD
D.L. = 5.5 PSF (10.0 PSF @ EXT.)
** L.L. = 10.0 PSF

* D.L. INCLUDES 12.0 PSF ALLOWANCE FOR FUTURE SOLAR SYS.

WIND
WIND SPEED = 110MPH
EXPOSURE = C

** NOT CONCURRENT w/ TOP CHORD LIVE LOAD.

TRUSS NOTES:

1. TOP CHORD SLOPE SHALL BE AS SPECIFIED BY ARCHITECT
2. EACH JOIST SHALL BE LEGIBLE MARKED ON THE BOTTOM CHORD WITH COMPANY NAME AND TRUSS IDENTIFICATION NUMBER.
3. A CERTIFICATION OF COMPLIANCE IS REQUIRED FOR EVERY TRUSS.
4. SEE ROOF PLAN FOR LOCATION AND WEIGHT OF MECH. EQUIPMENT SUPPORTED ON THE ROOF.
5. PROVIDE FULL DEPTH JOIST BLOCKING AT TRUSSES PERPENDICULAR TO SHEAR WALLS. BLOCKING SHALL EXTEND ENTIRE LENGTH OF SHEAR WALL.
6. PROVIDE BLOCKING AT PARAPET WALL BRACE ATTACHMENT / SUPPORT.
7. TRUSS BOTTOM CHORD SHALL BE DESIGNED FOR 250# DEAD LOAD SP/M/P/E EQUIP. ATTACHMENT ALONG ANYWHERE ALONG THE BOTTOM CHORD.
8. SEE SPECIAL GIRDER TRUSS LOAD TABLE (SHEET S-6.0) FOR ADDITIONAL LOADS AT TRUSSES SUPPORTTING TOWER WALLS.
9. G.E.T = GABLE END TRUSS, SEE PLANS FOR DRAG LOAD.
10. BOUNDRY NAIL (B.N.) ROOF SHEATHING TO DRAG TRUSS, UNO.
11. "ROT#" INDICATES DRAG TRUSS. DRAG TRUSS SHALL BE DESIGNED/SIZED FOR STRAP ATTACHMENT FOR STRAP SIZES SHOWN ON PLANS. TRUSS SHALL BE DESIGN FOR THE FOLLOWING MINIMUM DRAG CAPACITY:

DTW = PLF SAME AT SHEAR WALL BELOW
RDT2 = 2,000 lbs

GENERAL NOTES

CONTRACTOR TO CONFIRM BEAM SIZES, LOCATIONS, CALL OUTS, AND SHEAR WALL LOCATIONS. BRING TO THE ARCHITECTS ATTENTION ANY DISCREPANCIES FOUND PRIOR TO START OF CONSTRUCTION.

CONTRACTOR TO REVIEW W/ FRAMING CONTRACTOR THE FLOOR PLANS AND INTERIOR ELEVATION SHEET AND VERIFY ALL SOFFITS, ARCHES AND SPECIAL FRAMING CONDITIONS AND REPORT TO THE ARCHITECT ANY DISCREPANCIES PRIOR TO THE START OF CONSTRUCTION.

CONTRACTOR TO SCHEDULE FRAMING WALK-THROUGH WITH ENGINEER PRIOR TO INSULATION AND DRYWALL. ENGINEER TO GIVE CONTRACTOR WRITTEN RESPONSE OF WALK-THROUGH IN A FRAMING CERTIFICATION LETTER.

FLOOR/ROOF FRAMING NOTES

1. SEE GENERAL NOTES ON SHEET SN-1 AND SN-2
2. SEE STRUCTURAL STUD WALL SCHEDULE ON THIS DRAWING. SEE ARCHITECTURAL DRAWINGS FOR NON-LOAD BEARING PARTITIONS.
3. ALL BEARING STUD WALLS SHALL BE BLOCKED AT MID HEIGHT AS A MINIMUM. SEE SHEAR WALL SCHEDULE FOR PLYWOOD EDGE NAILING, SEE SHEET SN-1.
4. MARKS "◇" INDICATES PLYWOOD SHEAR WALLS WITH SHEATHING ON SIDE(S) OF WALL NOTED. SEE SHEAR WALL SCHEDULE ON SHEET SN-1.
MARKS "◇◇" INDICATES PLYWOOD SHEAR WALLS WITH SHEATHING ON BOTH SIDES OF WALL.
5. FOR ROOF DRAINS, OPENINGS, FASCIA DETAILS, ETC., SEE DRAWINGS OTHER THAN STRUCTURAL.
6. FRAME WALL OPENINGS PER DETAIL 1/SD2
7. ALL NON-BEARING INTERIOR WALLS PER DETAIL 3/SD2
8. TOP PLATE SPLICE PER DETAIL 2/SD2
9. –
10. TYPICAL FLOOR SHEATHING:
3/4" T&G UNDERLAYMENT GRADE PLYWOOD(P.1. 48/24),
B.N. PLY: 10d @ 6" o.c.
E.N. PLY: 10d @ 6" o.c.
F.N. PLY: 10d @ 12" o.c.
GLUE ALL CONTACT SURFACES.
11. TYPICAL ROOF SHEATHING:
5/8" CDX PLYWOOD(P.1. 32/16)
B.N. PLY: 8d @ 6" o.c.
E.N. PLY: 8d @ 6" o.c.
F.N. PLY: 8d @ 12" o.c.
PROVIDE PANEL EDGE CLIP MIDWAY BETWEEN EACH SUPPORT OR PROVIDE T&G PLYWOOD.
12. INDICATES SPECIAL ROOF/FLOOR DIAPHRAGM WITH ALL EDGES BLOCKED. SEE PLANS FOR SPECIAL NAILING REQUIREMENTS.
13. –
14. –
15. PROVIDE BLOCKING UNDER WALL PARALLEL TO JOIST PER DETAIL 17/SD2 TYP. U.N.O.
16. PROVIDE SOLID BLOCKING UNDER PERPENDICULAR WALLS PER DETAIL 17/SD2 TYP. U.N.O.
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18. REFER TO ARCHITECTURAL DRAWINGS FOR NON-STRUCTURAL SOFFIT FRAMING.
19. ABBREVIATIONS:

EFW = ENTIRE FACE OF WALL
G.T. = GIRDER TRUSS
J.T. = JACK TRUSS
D.T. = DRAG TRUSS w/ PLY SHTH'G B.N. (DRAG LOAD AS SPECIFIED)
FL = FLUSH BEAM
DP = DROP BEAM
M.S. = MULTIPLE STUDS (WIDTH TO MATCH BEAM, 2–2xSTUDS MIN.)
E.E. = SAME CONDITION OCCURS AT EACH END OF BEAM
D.G = DRAG JOIST/BEAM w/ PLY. SHT'G B.N.
BS = CONDITION OCCUR ON BOTH SIDES OF WALL, BEAM...etc
UNO = UNLESS NOTED OTHERWISE
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PA = POST ABOVE
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TRM = TRIMMER STUS / POSTS AT HEADERS
KP = KING POST
C.P. = CRIPPLE POST
= MULTIPLE STUD OR POST (2–2x STUDS, MIN. UNO.)
** = SPECIAL ALIGNMENT OF POST/HOLDOWN w/ CONDITION ABOVE OR BELOW.

20. PLUMBING, ELECTRICAL, MECHANICAL AND FIRESPRINKLER SYSTEMS SHALL BE DESIGNED AND INSTALLED TO ACCOMODATE WOOD SHRINKAGE (SETTLEMENT) AS FOLLOWS:
1st FLOOR: 1/4" TOTAL VERTICAL MOVEMENT
2nd FLOOR: 3/8" "
3rd FLOOR: 1/2" "
4th FLOOR: 5/8" "

POST ABREVATION

MARK	SIZE	MARK	SIZE
#P1	SEE NOTE #1	HS50	HSS 5x5x5/16
P44	4x4 POST	–	–
P46	4x6 POST	–	–
P48	4x8 POST	–	–
P86	6x6 POST	–	–
P88	6x8 POST	–	–

POST NOTES:
1. USE 2–2x STUDS MIN. (# = TOTAL NUMBER OF STUDS WHERE MORE THAN 2–2x STUDS ARE REQUIRED, Ex: 3P1 = 3–2x STUDS), STITCH NAIL EA. STUD w/ 16d @ 12" o.c.

HOLDOWN / VERT. STRAP SCHED.

MARK	SIZE	DETAIL
V40	MSTC40 (v)	32/SD2
V52	MSTC52 (v)	
V66	MSTC66 (v)	
V78	MSTC78 (v)	
V146	CS14 x 6'-0" (v)	
V128	CMST12 x 8'-0" (v)	
V168	CMSTC16 x 8'-0" (v)	
U8	HDU8 HLDN	32/SD2
U11	HDU11 HLDN	
U14	HDU14 HLDN	
D19	HD19	
H–#	INDICATES SPECIAL PATENTED HOLDDOWN SYSTEM. COORDINATE LOCATION WITH FRAMING AT LEVEL ABOVE (** INDICATES SPECIAL ALIGNMENT REQUIRED WITH ABOVE OR BELOW)	SEE SHEETS ATS–1 & ATS–2

LEGEND:

- DETAIL NUMBER / SHEET REFERENCE
- SHEAR WALL TYPE / SHEAR WALL LENGTH
- INDICATES FLOOR JOIST. SEE FLOOR JOIST SCHEDULE, U.N.O.
- INDICATES ROOF TRUSSES @ 24" o.c.
- V–##, U–##, H–## INDICATES HOLDOWN / VERTICAL STRAP. SEE HOLDOWN SCHEDULE FOR ADDITIONAL INFORMATION.
- POST OR TRIMMER STUDS AT BEAMS. USE 2–2x MIN. UNO.
- STEEL COLUMN
- WALL BELOW CURRENT FRAMING LEVEL
- WALL ABOVE CURRENT FRAMING LEVEL

CITY OF SANTA CLARITA
BUILDING & SAFETY

APPROVED
Under SCMC Titles
18, 19, 20, 21, 24, 25
Nov 3, 2020
R. Abdel-Messih

BLD19-00927

The approved plans must be available at the construction site at all times. Changes or alterations to approved plans shall not be made without written permission from the City of Santa Clarita Building & Safety Division. The approval of these plans shall not be construed to permit or approve any violation of the applicable codes, ordinances, or other laws.

HCP ENGINEERING
2466 REDBROOK DR.
CORONA, CA 92881
P: 951.738.0840
F: 951.738.1432
www.hcpengineeringco.com

DESIGN ENGINEERING

HAMPTON INN - SANTA CLARITA

NEWHALL RANCH RD

Santa Clarita, CA 91355

PLAN CHECK NO: BLD19-00927

SEAL

REGISTERED PROFESSIONAL ENGINEER
HITESH C. PATEL
No. C50389
Exp. 6-30-2019
STATE OF CALIFORNIA
CIVIL

10/31/20

4TH FLOOR FRAMING PLAN

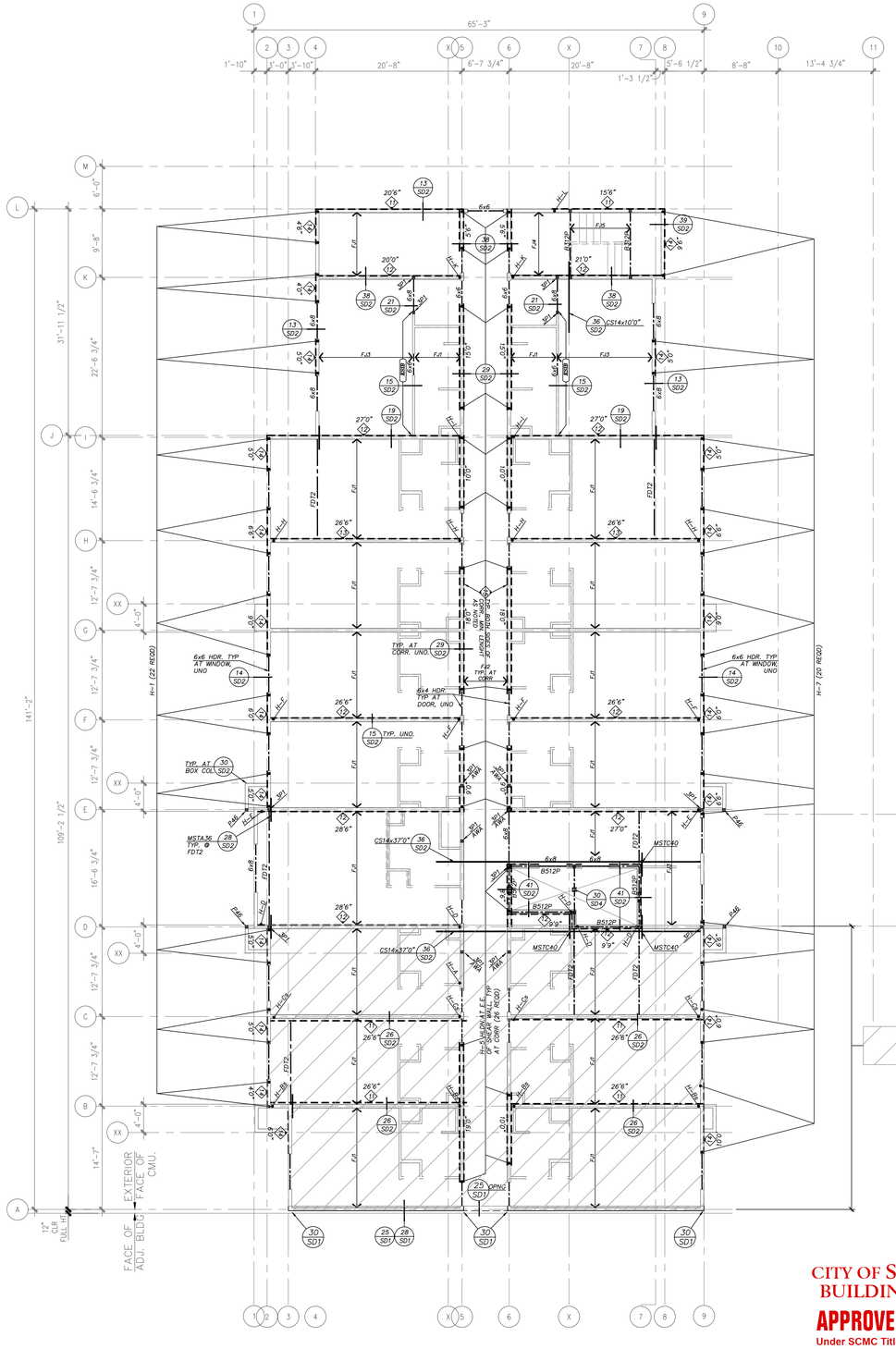
REVISION

9/12/19	1	PLAN CHECK #1
12/6/19	2	PLAN CHECK #2

PROJECT No.: 2018-14

PLOT DATE: October 31, 2020

SHEET: S-4.0



1 5TH FLOOR FRAMING PLAN
SCALE 1/8"=1'-0"

BEAM / HEADER SCHEDULE

MARK	SIZE	TRIMMERS/POST,UNO
4x4	4x4	2x4
4x6	4x6 / 6x4	2x4
4x8	4x8	2P1 or 4x4
4x10	4x10	2P1 or 4x4
4x12	4x12	3P1 or 4x4
6x6	6x6	2P1 or 4x6
6x8	6x8	2P1 or 4x6
6x10	6x10	3P1 or 4x6
6x12	6x12	3P1 or 4x6
B212L	1 3/4" x 12" LSL	2P1 or 4x4
B214L	1 3/4" x 14" LSL	2P1 or 4x4
B310P	3 1/2" x 9 1/2" PSL	3P1 or 4x4
B312P	3 1/2" x 11 7/8" PSL	4P1 or 4x6
B314P	3 1/2" x 14" PSL	4P1 or 4x6
B316P	3 1/2" x 16" PSL	4P1 or 4x6
B318P	3 1/2" x 18" PSL	4P1 or 4x6
B510P	5-1/4"x9 1/2" PSL	4P1 or 6x6
B512P	5-1/4"x11 7/8" PSL	6x6
B514P	5-1/4"x14" PSL	6x6
B516P	5-1/4"x16" PSL	6x6
B518P	5-1/4"x18" PSL	6x6
B520P	5-1/4"x20" PSL	6x6
B710P	7" x 9 1/2" PSL	8x4
B712P	7" x 11 7/8" PSL	8x6
B714P	7" x 14" PSL	8x6
B716P	7" x 16" PSL	8x6

BEAM/HEADER SCHEDULE NOTES:

1. TRIMMERS / POST SHALL BE INSTALLED AT EA. END OF BEAMS / HEADERS , U.N.O.

FLOOR JOIST SCHEDULE

MARK	SIZE
FJ-1	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 16'-6" (TYP. FLOOR LOAD)
FJ-2	2x8 @ 16" o.c. (Lmax= 6'-6") (CORR. LOAD)
FJ-3	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 14'-0" (TYP. FLOOR LOAD)
FJ-4	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 9'-8" (STAIR LOAD)
FJ-5	1 3/4"x14 LSL 1.55E STRINGERS @ 16" o.c. (STAIR LOAD), MAXIMUM SPAN LENGHT = 10'-0"

FLOOR JOIST NOTES:

FLOOR JOIST LOADS: (MIN.)

D.L.: TOP CHORD = 24.0 psf (SEE NOTE #1 & 2)
BOTT. CHORD = 8.0 psf (SEE NOTE #3)

L.L.: TOP CHORD = 40.0 psf TYP.
100.0 psf CORR.
125.0 psf STORAGE
60.0 psf BALCONY

NOTES:
1. PARTITION LOAD OF 10 psf (PARTITION HEIGHT) SHALL BE INCLUDED FOR NON-BEARING PARTITION AT PARTITION LOCATION.
2. ALL SUPERIMPOSED LOADS ABOVE JOIST SHALL BE INCLUDED IN JOIST DESIGN.
3. ADDITIONAL LOAD OF 250# DEAD LOAD ANY WHERE ALONG TRUSS BOTT. CHORD FOR FIRE SP/M/E/P EQUIP. ATTACHMENT.
4. DEFLECTION CRITERIA: TL = L/360, LL = L/480
5. DRAG TRUSSES (FDT#):
FDT2 = 2,000 lbs FDT6 = 6,000 lbs
FDT3 = 3,000 lbs FDT8 = 8,000 lbs
FDT4 = 4,000 lbs

WALL STUD FRAMING SCHEDULE

EXTERIOR WALLS	1st – 3x6 STUDS @ 16" o.c. DF#1 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	FIRE RATED STUDS, SEE NOTE #5
INTERIOR BEARING: (PARTY WALLS)	1st – 4x6 STUDS @ 16" o.c. 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	
INTERIOR BEARING: (CORR. AND ELEV. SHAFT WALLS)	1st – 3x6 STUDS @ 16" o.c., UNO. 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	
INTERIOR BEARING "XSB" 6" WALLS (KING SUITE BATH ROOM) (7"SB)	1st – 4x6 STUDS @ 16" o.c., UNO. 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	
TOWER WALLS	2x4 STUD @ 16" o.c. – MAX HT.= 10'-0" 2x6 STUD @ 16" o.c. – MAX HT.= 15'-0"	
INTERIOR NON-BEARING WALLS	2x4 STUD @ 16" o.c. – MAX HT.= 14'-0" 2x6 STUD @ 16" o.c. – MAX HT.= 20'-0"	

WALL STUD SCHEDULE NOTES:

1. ALL STRUCTURAL STUD LUMBER SHALL BE DOUGLAS FIR LARCH #2 OR BETTER.
2. ALL SILL PLATES IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED.
3. SEE SHEARWALL SCHEDULE ON SN-1 FOR ADDITIONAL WALL FRAMING REQUIREMENTS
4. USE 3x PRESSURE TREATED FOUNDATION SILL PLATE AT SHEAR WALLS.
5. EXTERIOR WALL FRAMING SHALL BE FIRE RETARDANT TREATED WOOD AND SHEATHING. NAILS AT FIRE RETARDANT TREATED EOOD SHALL BE GALVANIZED.

ROOF TRUSS NOTES

TOP CHORD
D.L. = 26.5 PSF*
L.L. = 20.0 PSF

BOTTOM CHORD
D.L. = 5.5 PSF (10.0 PSF @ EXT.)
** L.L. = 10.0 PSF

* D.L. INCLUDES 12.0 PSF ALLOWANCE FOR FUTURE SOLAR SYS.

WIND
WIND SPEED = 110MPH
EXPOSURE = C

** NOT CONCURRENT w/ TOP CHORD LIVE LOAD.

TRUSS NOTES:

1. TOP CHORD SLOPE SHALL BE AS SPECIFIED BY ARCHITECT
2. EACH JOIST SHALL BE LEGIBLE MARKED ON THE BOTTOM CHORD WITH COMPANY NAME AND TRUSS IDENTIFICATION NUMBER.
3. A CERTIFICATION OF COMPLIANCE IS REQUIRED FOR EVERY TRUSS.
4. SEE ROOF PLAN FOR LOCATION AND WEIGHT OF MECH. EQUIPMENT SUPPORTED ON THE ROOF.
5. PROVIDE FULL DEPTH JOIST BLOCKING AT TRUSSES PERPENDICULAR TO SHEAR WALLS. BLOCKING SHALL EXTEND ENTIRE LENGTH OF SHEAR WALL.
6. PROVIDE BLOCKING AT PARAPET WALL BRACE ATTACHMENT / SUPPORT.
7. TRUSS BOTTOM CHORD SHALL BE DESIGNED FOR 250# DEAD LOAD SP/M/P/E EQUIP. ATTACHMENT ALONG ANYWHERE ALONG THE BOTTOM CHORD.
8. SEE SPECIAL GIRDER TRUSS LOAD TABLE (SHEET S-6.0) FOR ADDITIONAL LOADS AT TRUSSES SUPPORTTING TOWER WALLS.
9. G.E.T = GABLE END TRUSS, SEE PLANS FOR DRAG LOAD.
10. BOUNDRY NAIL (B.N.) ROOF SHEATHING TO DRAG TRUSS, UNO.
11. "#DT#" INDICATES DRAG TRUSS. DRAG TRUSS SHALL BE DESIGNED/SIZED FOR STRAP ATTACHMENT FOR STRAP SIZES SHOWN ON PLANS. TRUSS SHALL BE DESIGN FOR THE FOLLOWING MINIMUM DRAG CAPACITY:

DTW = PLF SAME AT SHEAR WALL BELOW
RDT2 = 2,000 lbs

GENERAL NOTES

CONTRACTOR TO CONFIRM BEAM SIZES, LOCATIONS, CALL OUTS, AND SHEAR WALL LOCATIONS. BRING TO THE ARCHITECTS ATTENTION ANY DISCREPANCIES FOUND PRIOR TO START OF CONSTRUCTION.

CONTRACTOR TO REVIEW W/ FRAMING CONTRACTOR THE FLOOR PLANS AND INTERIOR ELEVATION SHEET AND VERIFY ALL SOFFITS, ARCHES AND SPECIAL FRAMING CONDITIONS AND REPORT TO THE ARCHITECT ANY DISCREPANCIES PRIOR TO THE START OF CONSTRUCTION.

CONTRACTOR TO SCHEDULE FRAMING WALK-THROUGH WITH ENGINEER PRIOR TO INSULATION AND DRYWALL. ENGINEER TO GIVE CONTRACTOR WRITTEN RESPONSE OF WALK-THROUGH IN A FRAMING CERTIFICATION LETTER.

FLOOR/ROOF FRAMING NOTES

1. SEE GENERAL NOTES ON SHEET SN-1 AND SN-2
2. SEE STRUCTURAL STUD WALL SCHEDULE ON THIS DRAWING. SEE ARCHITECTURAL DRAWINGS FOR NON-LOAD BEARING PARTITIONS.
3. ALL BEARING STUD WALLS SHALL BE BLOCKED AT MID HEIGHT AS A MINIMUM. SEE SHEAR WALL SCHEDULE FOR PLYWOOD EDGE NAILING, SEE SHEET SN-1.
4. MARKS "◇" INDICATES PLYWOOD SHEAR WALLS WITH SHEATHING ON SIDE(S) OF WALL NOTED. SEE SHEAR WALL SCHEDULE ON SHEET SN-1.
MARKS "◇" INDICATES PLYWOOD SHEAR WALLS WITH SHEATHING ON BOTH SIDES OF WALL.
5. FOR ROOF DRAINS, OPENINGS, FASCIA DETAILS, ETC., SEE DRAWINGS OTHER THAN STRUCTURAL.
6. FRAME WALL OPENINGS PER DETAIL 1/SD2
7. ALL NON-BEARING INTERIOR WALLS PER DETAIL 3/SD2
8. TOP PLATE SPLICE PER DETAIL 2/SD2
9. –
10. TYPICAL FLOOR SHEATHING:
3/4" T&G UNDERLAYMENT GRADE PLYWOOD(P.1. 48/24),
B.N. PLY: 10d @ 6" o.c.
E.N. PLY: 10d @ 6" o.c.
F.N. PLY: 10d @ 12" o.c.
GLUE ALL CONTACT SURFACES.
11. TYPICAL ROOF SHEATHING:
5/8" CDX PLYWOOD(P.1. 32/16)
B.N. PLY: 8d @ 6" o.c.
E.N. PLY: 8d @ 6" o.c.
F.N. PLY: 8d @ 12" o.c.
PROVIDE PANEL EDGE CLIP MIDWAY BETWEEN EACH SUPPORT OR PROVIDE T&G PLYWOOD.
12. INDICATES SPECIAL ROOF/FLOOR DIAPHRAGM WITH ALL EDGES BLOCKED. SEE PLANS FOR SPECIAL NAILING REQUIREMENTS.
13. –
14. –
15. PROVIDE BLOCKING UNDER WALL PARALLEL TO JOIST PER DETAIL 17/SD2 TYP. U.N.O.
16. PROVIDE SOLID BLOCKING UNDER PERPENDICULAR WALLS PER DETAIL 17/SD2 TYP. U.N.O.
17. NON-SHEAR WALLS ADJACENT TO AND PARALLEL WITH SHEAR WALLS SHALL RECEIVE PLYWOOD SHEATHING TO PROVIDE SMOOTH TRANSITION FROM SHEAR WALLS TO NON-SHEAR WALLS.
18. REFER TO ARCHITECTURAL DRAWINGS FOR NON-STRUCTURAL SOFFIT FRAMING.
19. ABBREVIATIONS:

EFW = ENTIRE FACE OF WALL
G.T. = GIRDER TRUSS
J.T. = JACK TRUSS
D.T. = DRAG TRUSS w/ PLY SHTH'G B.N. (DRAG LOAD AS SPECIFIED)
FL = FLUSH BEAM
DP = DROP BEAM
M.S. = MULTIPLE STUDS (WIDTH TO MATCH BEAM, 2–2xSTUDS MIN.)
E.E. = SAME CONDITION OCCURS AT EACH END OF BEAM
D.G = DRAG JOIST/BEAM w/ PLY. SHT'G B.N.
BS = CONDITION OCCUR ON BOTH SIDES OF WALL, BEAM...etc
UNO = UNLESS NOTED OTHERWISE
AWA = ALIGN WITH ABOVE
PA = POST ABOVE
GET = GABLE END TRUSS (DRAG TRUSS WHERE NOTED)
TRM = TRIMMER STUS / POSTS AT HEADERS
KP = KING POST
C.P. = CRIPPLE POST
= MULTIPLE STUD OR POST (2–2x STUDS, MIN. UNO.)
** = SPECIAL ALIGNMENT OF POST/HOLDOWN w/ CONDITION ABOVE OR BELOW.

20. PLUMBING, ELECTRICAL, MECHANICAL AND FIRESPRINKLER SYSTEMS SHALL BE DESIGNED AND INSTALLED TO ACCOMODATE WOOD SHRINKAGE (SETTLEMENT) AS FOLLOWS:
1st FLOOR: 1/4" TOTAL VERTICAL MOVEMENT
2nd FLOOR: 3/8" "
3rd FLOOR: 1/2" "
4th FLOOR: 5/8" "

POST ABREVATION

MARK	SIZE	MARK	SIZE
#P1	SEE NOTE #1	HS50	HSS 5x5x5/16
P44	4x4 POST	–	–
P46	4x6 POST	–	–
P48	4x8 POST	–	–
P86	6x6 POST	–	–
P88	6x8 POST	–	–

POST NOTES:
1. USE 2–2x STUDS MIN. (# = TOTAL NUMBER OF STUDS WHERE MORE THAN 2–2x STUDS ARE REQUIRED, Ex: 3P1 = 3–2x STUDS), STITCH NAIL EA. STUD w/ 16d @ 12" o.c.

HOLDOWN / VERT. STRAP SCHED.

MARK	SIZE	DETAIL
V40	MSTC40 (v)	32/SD2
V52	MSTC52 (v)	
V66	MSTC66 (v)	
V78	MSTC78 (v)	
V146	CS14 x 6'-0" (v)	
V128	CMST12 x 8'-0" (v)	
V168	CMSTC16 x 8'-0" (v)	
U8	HDU8 HLDN	32/SD2
U11	HDU11 HLDN	
U14	HDU14 HLDN	
D19	HD19	
H-#	INDICATES SPECIAL PATENTED HOLDDOWN SYSTEM. COORDINATE LOCATION WITH FRAMING AT LEVEL ABOVE (** INDICATES SPECIAL ALIGNMENT REQUIRED WITH ABOVE OR BELOW)	SEE SHEETS ATS-1 & ATS-2

LEGEND:

- 4/SH1 — DETAIL NUMBER / SHEET REFERENCE
- 2/XX — SHEAR WALL TYPE / SHEAR WALL LENGTH
- XX — INDICATES FLOOR JOIST. SEE FLOOR JOIST SCHEDULE, U.N.O.
- XX — INDICATES ROOF TRUSSES @ 24" o.c.
- V-##, U-##, H-## — INDICATES HOLDOWN / VERTICAL STRAP. SEE HOLDOWN SCHEDULE FOR ADDITIONAL INFORMATION.
- ⊠ POST OR TRIMMER STUDS AT BEAMS. USE 2–2x MIN. UNO.
- □ STEEL COLUMN
- WALL BELOW CURRENT FRAMING LEVEL
- WALL ABOVE CURRENT FRAMING LEVEL

CITY OF SANTA CLARITA
BUILDING & SAFETY

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DESIGN ENGINEERING

HAMPTON INN - SANTA CLARITA
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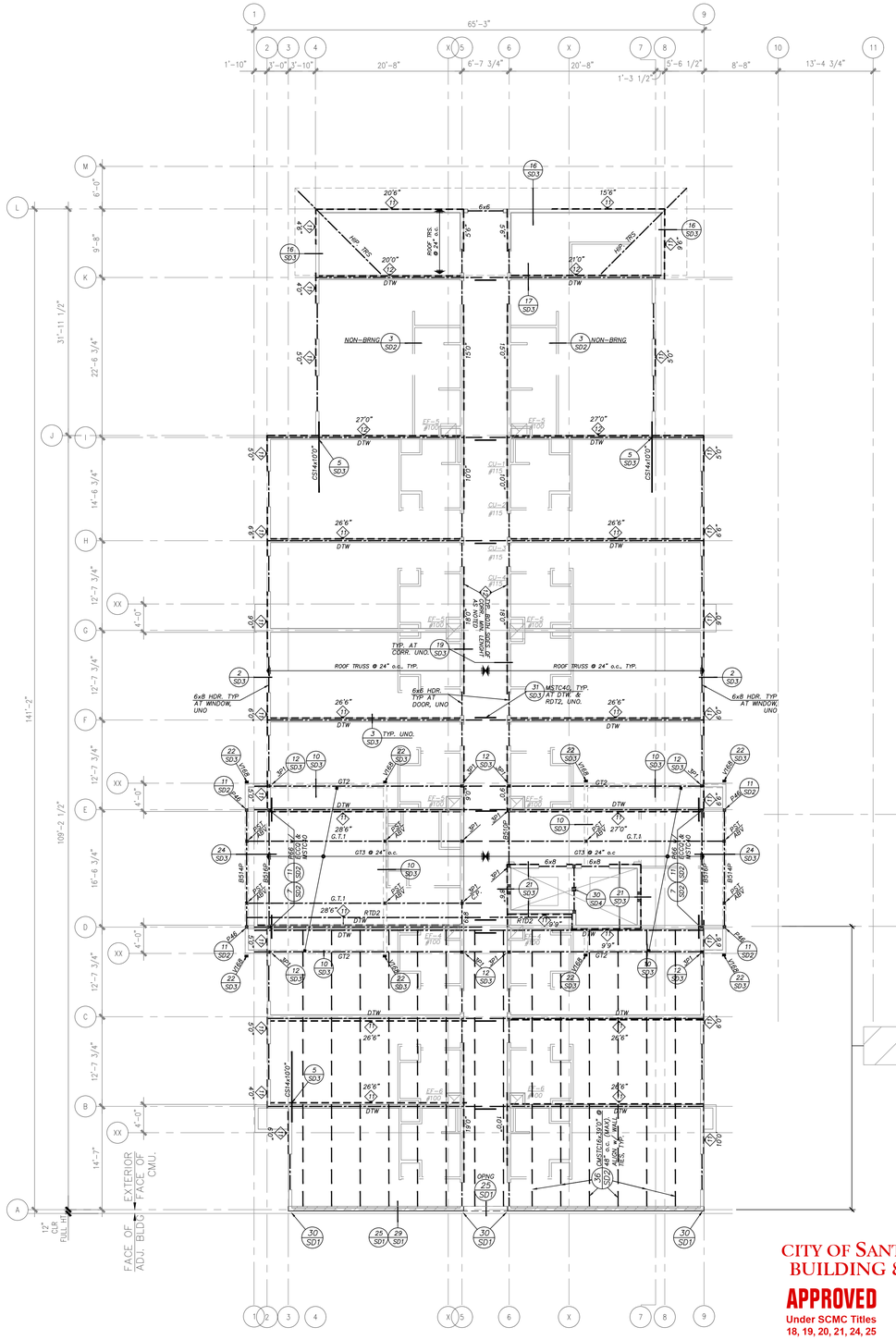
CIVIL
10/31/20

5TH FLOOR FRAMING PLAN

REVISION

9/12/19	1	PLAN CHECK #1
12/6/19	2	PLAN CHECK #2

PROJECT No.: 2018-14
PLOT DATE: October 31, 2020
SHEET: **S-5.0**



1 ROOF FRAMING PLAN
SCALE 1/8"=1'-0"

BEAM / HEADER SCHEDULE

MARK	SIZE	TRIMMERS/POST,UNO
4x4	4x4	2x4
4x6	4x6 / 6x4	2x4
4x8	4x8	2P1 or 4x4
4x10	4x10	2P1 or 4x4
4x12	4x12	3P1 or 4x4
6x6	6x6	2P1 or 4x6
6x8	6x8	2P1 or 4x6
6x10	6x10	3P1 or 4x6
6x12	6x12	3P1 or 4x6
B212L	1 3/4" x 12" LSL	2P1 or 4x4
B214L	1 3/4" x 14" LSL	2P1 or 4x4
B310P	3 1/2" x 9 1/2" PSL	3P1 or 4x4
B312P	3 1/2" x 11 7/8" PSL	4P1 or 4x6
B314P	3 1/2" x 14" PSL	4P1 or 4x6
B316P	3 1/2" x 16" PSL	4P1 or 4x6
B318P	3 1/2" x 18" PSL	4P1 or 4x6
B510P	5-1/4"x9 1/2" PSL	4P1 or 6x6
B512P	5-1/4"x11 7/8" PSL	6x6
B514P	5-1/4"x14" PSL	6x6
B516P	5-1/4"x16" PSL	6x6
B518P	5-1/4"x18" PSL	6x6
B520P	5-1/4"x20" PSL	6x6
B710P	7" x 9 1/2" PSL	8x4
B712P	7" x 11 7/8" PSL	8x6
B714P	7" x 14" PSL	8x6
B716P	7" x 16" PSL	8x6

BEAM/HEADER SCHEDULE NOTES:

1. TRIMMERS / POST SHALL BE INSTALLED AT EA. END OF BEAMS / HEADERS , U.N.O.

FLOOR JOIST SCHEDULE

MARK	SIZE
FJ-1	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 16'-6" (TYP. FLOOR LOAD)
FJ-2	2x8 @ 16" o.c. (Lmax= 6'-6") (CORR. LOAD)
FJ-3	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 14'-0" (TYP. LOAD)
FJ-4	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 9'-8" (STAIR LOAD)
FJ-5	1 3/4"x14 LSL 1.55E STRINGERS @ 16" o.c. (STAIR LOAD), MAXIMUM SPAN LENGHT = 10'-0"

FLOOR JOIST NOTES:

FLOOR JOIST LOADS: (MIN.)
D.L.: TOP CHORD = 24.0 psf (SEE NOTE #1 & 2)
BOTT. CHORD = 8.0 psf (SEE NOTE #3)

L.L.: TOP CHORD = 40.0 psf TYP.
100.0 psf CORR.
125.0 psf STORAGE
60.0 psf BALCONY

NOTES:
1. PARTITION LOAD OF 10 psf (PARTITION HEIGHT) SHALL BE INCLUDED FOR NON-BEARING PARTITION AT PARTITION LOCATION.
2. ALL SUPERIMPOSED LOADS ABOVE JOIST SHALL BE INCLUDED IN JOIST DESIGN.
3. ADDITIONAL LOAD OF 250# DEAD LOAD ANY WHERE ALONG TRUSS BOTT. CHORD FOR FIRE SP/M/E/P EQUIP. ATTACHMENT.
4. DEFLECTION CRITERIA: TL = L/360, LL = L/480
5. DRAG TRUSSES (FDT#):
FDT2 = 2,000 lbs FDT6 = 6,000 lbs
FDT3 = 3,000 lbs FDT8 = 8,000 lbs
FDT4 = 4,000 lbs

WALL STUD FRAMING SCHEDULE

EXTERIOR WALLS	1st – 3x6 STUDS @ 16" o.c. DF#1 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	FIRE RATED STUDS, SEE NOTE #5
INTERIOR BEARING: (PARTY WALLS)	1st – 4x6 STUDS @ 16" o.c. 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	
INTERIOR BEARING: (CORR. AND ELEV. SHAFT WALLS)	1st – 3x6 STUDS @ 16" o.c., UNO. 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	
INTERIOR BEARING "XSB" 6" WALLS (KING SUITE BATH ROOM)	1st – 4x6 STUDS @ 16" o.c., UNO. 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	(7.5"@ 1ST)
TOWER WALLS	2x4 STUD @ 16" o.c. – MAX HT.= 10'-0" 2x6 STUD @ 16" o.c. – MAX HT.= 15'-0"	
INTERIOR NON-BEARING WALLS	2x4 STUD @ 16" o.c. – MAX HT.= 14'-0" 2x6 STUD @ 16" o.c. – MAX HT.= 20'-0"	

WALL STUD SCHEDULE NOTES:

1. ALL STRUCTURAL STUD LUMBER SHALL BE DOUGLAS FIR LARCH #2 OR BETTER.
2. ALL SILL PLATES IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED.
3. SEE SHEARWALL SCHEDULE ON SN-1 FOR ADDITIONAL WALL FRAMING REQUIREMENTS
4. USE 3x PRESSURE TREATED FOUNDATION SILL PLATE AT SHEAR WALLS.
5. EXTERIOR WALL FRAMING SHALL BE FIRE RETARDANT TREATED WOOD AND SHEATHING. NAILS AT FIRE RETARDANT TREATED EOOD SHALL BE GALVANIZED.

ROOF TRUSS NOTES

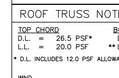
TOP CHORD
D.L. = 26.5 PSF*
L.L. = 20.0 PSF

BOTTOM CHORD
D.L. = 5.5 PSF (10.0 PSF @ EXT.)
**L.L. = 10.0 PSF

* D.L. INCLUDES 12.0 PSF ALLOWANCE FOR FUTURE SOLAR SYS.

WND
WIND SPEED = 110MPH
EXPOSURE = C

** NOT CONCURRENT w/ TOP CHORD LIVE LOAD.



TRUSS NOTES:

1. TOP CHORD SLOPE SHALL BE AS SPECIFIED BY ARCHITECT
2. EACH JOIST SHALL BE LEGIBLE MARKED ON THE BOTTOM CHORD WITH COMPANY NAME AND TRUSS IDENTIFICATION NUMBER.
3. A CERTIFICATION OF COMPLIANCE IS REQUIRED FOR EVERY TRUSS.
4. SEE ROOF PLAN FOR LOCATION AND WEIGHT OF MECH. EQUIPMENT SUPPORTED ON THE ROOF.
5. PROVIDE FULL DEPTH JOIST BLOCKING AT TRUSSES PERPENDICULAR TO SHEAR WALLS. BLOCKING SHALL EXTEND ENTIRE LENGTH OF SHEAR WALL.
6. PROVIDE BLOCKING AT PARAPET WALL BRACE ATTACHMENT / SUPPORT.
7. TRUSS BOTTOM CHORD SHALL BE DESIGNED FOR 250# DEAD LOAD SP/M/P/E EQUIP. ATTACHMENT ALONG ANYWHERE ALONG THE BOTTOM CHORD.
8. SEE SPECIAL GIRDER TRUSS LOAD TABLE (SHEET S-6.0) FOR ADDITIONAL LOADS AT TRUSSES SUPPORTTING TOWER WALLS.
9. G.T.= GABLE END TRUSS, SEE PLANS FOR DRAG LOAD.
10. BOUNDRY NAIL (B.N.) ROOF SHEATHING TO DRAG TRUSS, UNO.
11. "RDT#" INDICATES DRAG TRUSS. DRAG TRUSS SHALL BE DESIGNED/SIZED FOR STRAP ATTACHMENT FOR STRAP SIZES SHOWN ON PLANS. TRUSS SHALL BE DESIGN FOR THE FOLLOWING MINIMUM DRAG CAPACITY:

DTW = PLF SAME AT SHEAR WALL BELOW
RDT2 = 2,000 lbs

GENERAL NOTES

CONTRACTOR TO CONFIRM BEAM SIZES, LOCATIONS, CALL OUTS, AND SHEAR WALL LOCATIONS. BRING TO THE ARCHITECTS ATTENTION ANY DISCREPANCIES FOUND PRIOR TO START OF CONSTRUCTION.

CONTRACTOR TO REVIEW W/ FRAMING CONTRACTOR THE FLOOR PLANS AND INTERIOR ELEVATION SHEET AND VERIFY ALL SOFFITS, ARCHES AND SPECIAL FRAMING CONDITIONS AND REPORT TO THE ARCHITECT ANY DISCREPANCIES PRIOR TO THE START OF CONSTRUCTION.

CONTRACTOR TO SCHEDULE FRAMING WALK-THROUGH WITH ENGINEER PRIOR TO INSULATION AND DRYWALL. ENGINEER TO GIVE CONTRACTOR WRITTEN RESPONSE OF WALK-THROUGH IN A FRAMING CERTIFICATION LETTER.

FLOOR/ROOF FRAMING NOTES

1. SEE GENERAL NOTES ON SHEET SN-1 AND SN-2
2. SEE STRUCTURAL STUD WALL SCHEDULE ON THIS DRAWING. SEE ARCHITECTURAL DRAWINGS FOR NON-LOAD BEARING PARTITIONS.
3. ALL BEARING STUD WALLS SHALL BE BLOCKED AT MID HEIGHT AS A MINIMUM. SEE SHEAR WALL SCHEDULE FOR PLYWOOD EDGE NAILING, SEE SHEET SN-1.
4. MARKS "◇" INDICATES PLYWOOD SHEAR WALLS WITH SHEATHING ON SIDE(S) OF WALL NOTED. SEE SHEAR WALL SCHEDULE ON SHEET SN-1.
MARKS "◇◇" INDICATES PLYWOOD SHEAR WALLS WITH SHEATHING ON BOTH SIDES OF WALL.
5. FOR ROOF DRAINS, OPENINGS, FASCIA DETAILS, ETC., SEE DRAWINGS OTHER THAN STRUCTURAL.
6. FRAME WALL OPENINGS PER DETAIL 1/SD2
7. ALL NON-BEARING INTERIOR WALLS PER DETAIL 2/SD2
8. TOP PLATE SPLICE PER DETAIL 2/SD2
9. –
10. TYPICAL FLOOR SHEATHING:
3/4" T&G UNDERLAYMENT GRADE PLYWOOD(P.I. 48/24),
B.N. PLY: 10d @ 6" o.c.
E.N. PLY: 10d @ 6" o.c.
F.N. PLY: 10d @ 12" o.c.
GLUE ALL CONTACT SURFACES.
11. TYPICAL ROOF SHEATHING:
5/8" CDX PLYWOOD(P.I. 32/16)
B.N. PLY: 8d @ 6" o.c.
E.N. PLY: 8d @ 6" o.c.
F.N. PLY: 8d @ 12" o.c.
PROVIDE PANEL EDGE CLIP MIDWAY BETWEEN EACH SUPPORT OR PROVIDE T&G PLYWOOD.
12. INDICATES SPECIAL ROOF/FLOOR DIAPHRAGM WITH ALL EDGES BLOCKED. SEE PLANS FOR SPECIAL NAILING REQUIREMENTS.
13.
14.
15. PROVIDE BLOCKING UNDER WALL PARALLEL TO JOIST PER DETAIL 17/SD2 TYP. U.N.O.
16. PROVIDE SOLID BLOCKING UNDER PERPENDICULAR WALLS PER DETAIL 17/SD2 TYP. U.N.O.
17. NON-SHEAR WALLS ADJACENT TO AND PARALLEL WITH SHEAR WALLS SHALL RECEIVE PLYWOOD SHEATHING TO PROVIDE SMOOTH TRANSITION FROM SHEAR WALLS TO NON-SHEAR WALLS.
18. REFER TO ARCHITECTURAL DRAWINGS FOR NON-STRUCTURAL SOFFIT FRAMING.
19. ABBREVIATIONS:

EFW =	ENTIRE FACE OF WALL
G.T. =	GIRDER TRUSS
J.T. =	JACK TRUSS
D.T. =	DRAG TRUSS w/ PLY SHTH'G B.N. (DRAG LOAD AS SPECIFIED)
FL =	FLUSH BEAM
DP =	DROP BEAM
M.S. =	MULTIPLE STUDS (WIDTH TO MATCH BEAM, 2-2xSTUDS MIN.)
E.E. =	SAME CONDITION OCCURS AT EACH END OF BEAM
D.G =	DRAG JOIST/BEAM w/ PLY. SHT'G B.N.
BS =	CONDITION OCCUR ON BOTH SIDES OF WALL, BEAM...etc
UNO =	UNLESS NOTED OTHERWISE
AWA =	ALIGN WITH ABOVE
PA =	POST ABOVE
GET =	GABLE END TRUSS (DRAG TRUSS WHERE NOTED)
TRM =	TRIMMER STUS / POSTS AT HEADERS
KP =	KING POST
C.P. =	CRIPPLE POST
## =	MULTIPLE STUD OR POST (2-2x STUDS, MIN. UNO.)
** =	SPECIAL ALIGNMENT OF POST/HOLDOWN w/ CONDITION ABOVE OR BELOW.

20. PLUMBING, ELECTRICAL, MECHANICAL AND FIRESPRINKLER SYSTEMS SHALL BE DESIGNED AND INSTALLED TO ACCOMODATE WOOD SHRINKAGE (SETTLEMENT) AS FOLLOWS:
1st FLOOR: 1/4" TOTAL VERTICAL MOVEMENT
2nd FLOOR: 3/8" "
3rd FLOOR: 1/2" "
4th FLOOR: 5/8" "

POST ABREVATION

MARK	SIZE	MARK	SIZE
#P1	SEE NOTE #1	HS50	HSS 5x5x5/16
P44	4x4 POST	–	–
P46	4x6 POST	–	–
P48	4x8 POST	–	–
P86	6x6 POST	–	–
P88	6x8 POST	–	–

POST NOTES:
1. USE 2-2x STUDS MIN. (# = TOTAL NUMBER OF STUDS WHERE MORE THAN 2-2x STUDS ARE REQUIRED, Ex: 3P1 = 3-2x STUDS), STITCH NAIL EA. STUD w/ 16d @ 12" o.c.

HOLDOWN / VERT. STRAP SCHED.

MARK	SIZE	DETAIL
V40	MSTC40 (v)	32/SD2
V52	MSTC52 (v)	
V66	MSTC66 (v)	
V78	MSTC78 (v)	
V146	CS14 x 6'-0" (v)	
V128	CMSTC12 x 8'-0" (v)	
V168	CMSTC16 x 8'-0" (v)	
U8	HDU8 HLDN	32/SD2
U11	HDU11 HLDN	
U14	HDU14 HLDN	
D19	HD19	
H-#	INDICATES SPECIAL PATENTED HOLDDOWN SYSTEM. COORDINATE LOCATION WITH FRAMING AT LEVEL ABOVE (** INDICATES SPECIAL ALIGNMENT REQUIRED WITH ABOVE OR BELOW)	SEE SHEETS ATS-1 & ATS-2

SPECIAL GIRDER TRUSS LOADS NOTES:

THE FOLLOWING LOADS SHALL ADDED AS SUPERIMPOSED LOADS IN ADDITION TO TRUSS STANDARD LOADS

GT1: POINT LOAD (POST ABOVE)
D.L.= 1,700 #
L.L.= 1,300 #

GT2: WALL LOAD (WALL ABOVE)
D.L.= 310 PLF
L.L.= 160 PLF

GT3: POINT LOAD (WALL ABOVE)
D.L.= 1,000 #
L.L.= 550 #

LEGEND:

- DETAIL NUMBER / SHEET REFERENCE
- SHEAR WALL TYPE / SHEAR WALL LENGTH
- INDICATES FLOOR JOIST. SEE FLOOR JOIST SCHEDULE, U.N.O.
- INDICATES ROOF TRUSSES @ 24" o.c.
- INDICATES HOLDOWN / VERTICAL STRAP. SEE HOLDOWN SCHEDULE FOR ADDITIONAL INFORMATION.
- POST OR TRIMMER STUDS AT BEAMS. USE 2-2x MIN. UNO.
- STEEL COLUMN
- WALL BELOW CURRENT FRAMING LEVEL
- WALL ABOVE CURRENT FRAMING LEVEL

CITY OF SANTA CLARITA
BUILDING & SAFETY
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Nov 3, 2020
R. Abdel-Messih
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HCP ENGINEERING
2466 BEDROCK DR.
CORONA, CA 92881
P: 951.738.0840
F: 951.738.1432
www.hcpengineeringco.com

HAMPTON INN - SANTA CLARITA

NEWHALL RANCH RD

Santa Clarita, CA 91355

PLAN CHECK NO: BLD19-00927

SEAL
REGISTERED PROFESSIONAL ENGINEER
HITESH C. PATEL
No. C50389
Exp. 6-30-2019
STATE OF CALIFORNIA
CIVIL
10/31/20

ROOF FRAMING PLAN

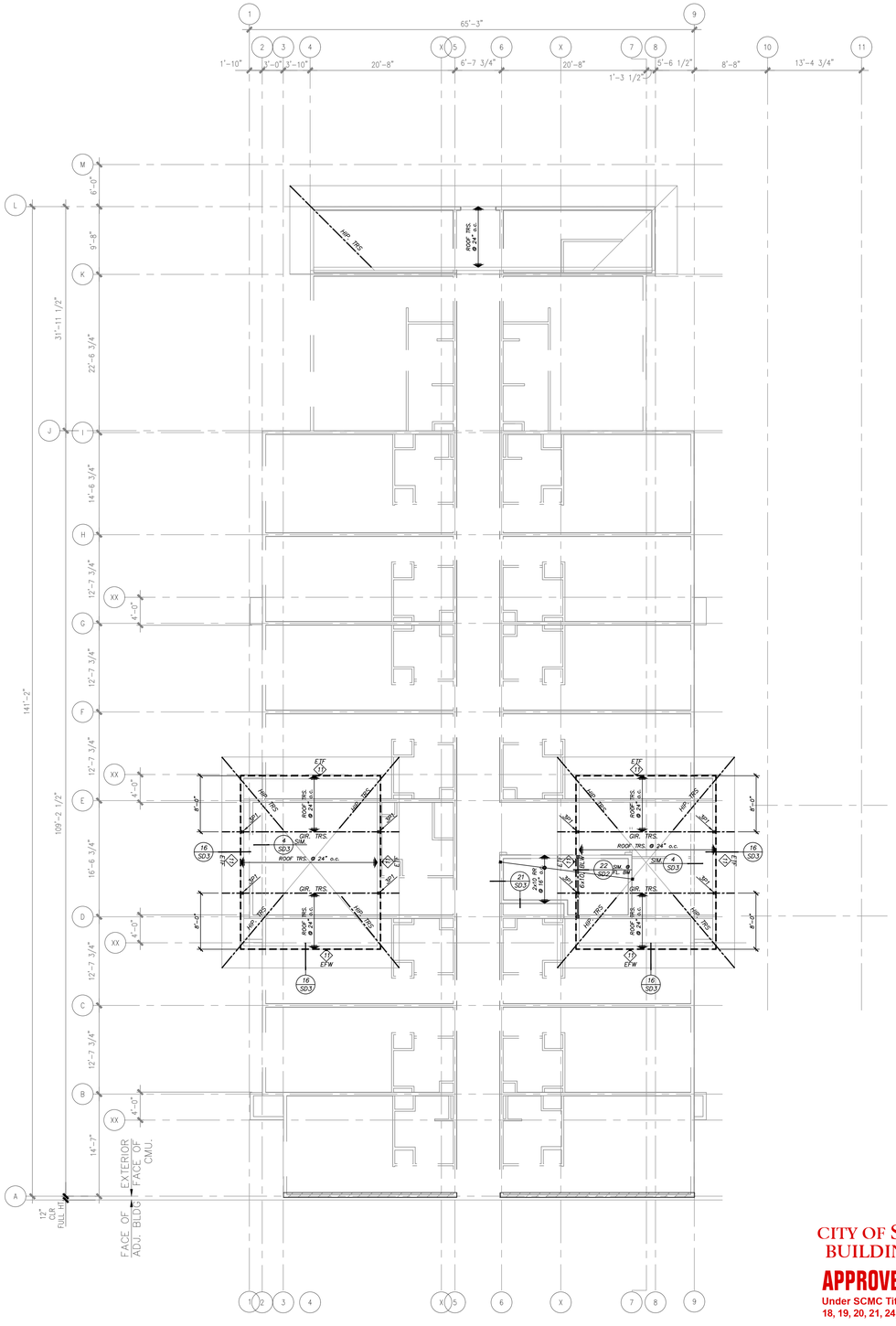
REVISION

9/12/19	1	PLAN CHECK #1
12/6/19	2	PLAN CHECK #2

PROJECT No.: 2018-14

PLOT DATE: October 31, 2020

SHEET: S-6.0



1 TOWER ROOF FRAMING PLAN
SCALE 1/8"=1'-0"

BEAM / HEADER SCHEDULE

MARK	SIZE	TRIMMERS/POST,UNO
4x4	4x4	2x4
4x6	4x6 / 6x4	2x4
4x8	4x8	2P1 or 4x4
4x10	4x10	2P1 or 4x4
4x12	4x12	3P1 or 4x4
6x6	6x6	2P1 or 4x6
6x8	6x8	2P1 or 4x6
6x10	6x10	3P1 or 4x6
6x12	6x12	3P1 or 4x6
B212L	1 3/4" x 12" LSL	2P1 or 4x4
B214L	1 3/4" x 14" LSL	2P1 or 4x4
B310P	3 1/2" x 9 1/2" PSL	3P1 or 4x4
B312P	3 1/2" x 11 7/8" PSL	4P1 or 4x6
B314P	3 1/2" x 14" PSL	4P1 or 4x6
B316P	3 1/2" x 16" PSL	4P1 or 4x6
B318P	3 1/2" x 18" PSL	4P1 or 4x6
B510P	5-1/4"x9 1/2" PSL	4P1 or 6x6
B512P	5-1/4"x11 7/8" PSL	6x6
B514P	5-1/4"x14" PSL	6x6
B516P	5-1/4"x16" PSL	6x6
B518P	5-1/4"x18" PSL	6x6
B520P	5-1/4"x20" PSL	6x6
B710P	7" x 9 1/2" PSL	8x4
B712P	7" x 11 7/8" PSL	8x6
B714P	7" x 14" PSL	8x6
B716P	7" x 16" PSL	8x6

BEAM/HEADER SCHEDULE NOTES:

1. TRIMMERS / POST SHALL BE INSTALLED AT EA. END OF BEAMS / HEADERS , U.N.O.

FLOOR JOIST SCHEDULE

MARK	SIZE
FJ-1	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 16'-6" (TYP. FLOOR LOAD)
FJ-2	2x8 @ 16" o.c. (Lmax= 6'-6") (CORR. LOAD)
FJ-3	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 14'-0" (FLOOR LOAD)
FJ-4	14" DEEP OPEN-WEB FLOOR TRUSS @ 16" o.c. (MAX), MAXIMUM SPAN LENGHT = 9'-8" (STAIR LOAD)
FJ-5	1 3/4"x14 LSL 1.55E STRINGERS @ 16" o.c. (STAIR LOAD), MAXIMUM SPAN LENGHT = 10'-0"

FLOOR JOIST NOTES:

FLOOR JOIST LOADS: (MIN.)

D.L.: TOP CHORD = 24.0 psf (SEE NOTE #1 & 2)
BOTT. CHORD = 8.0 psf (SEE NOTE #3)

L.L.: TOP CHORD = 40.0 psf TYP.
100.0 psf CORR.
125.0 psf STORAGE
60.0 psf BALCONY

NOTES:
1. PARTITION LOAD OF 10 psf (PARTITION HEIGHT) SHALL BE INCLUDED FOR NON-BEARING PARTITION AT PARTITION LOCATION.
2. ALL SUPERIMPOSED LOADS ABOVE JOIST SHALL BE INCLUDED IN JOIST DESIGN.
3. ADDITIONAL LOAD OF 250# DEAD LOAD ANY WHERE ALONG TRUSS BOTT. CHORD FOR FIRE SP/M/E/P EQUIP. ATTACHMENT.
4. DEFLECTION CRITERIA: TL = L/360, LL = L/480
5. DRAG TRUSSES (FDT#):
FDT2 = 2,000 lbs FDT6 = 6,000 lbs
FDT3 = 3,000 lbs FDT8 = 8,000 lbs
FDT4 = 4,000 lbs

WALL STUD FRAMING SCHEDULE

EXTERIOR WALLS	1st – 3x6 STUDS @ 16" o.c. DF#1 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	FIRE RATED STUDS, SEE NOTE #5
INTERIOR BEARING: (PARTY WALLS)	1st – 4x6 STUDS @ 16" o.c. 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	
INTERIOR BEARING: (CORR. AND ELEV. SHAFT WALLS)	1st – 3x6 STUDS @ 16" o.c., UNO. 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	
INTERIOR BEARING "XSB", 6" WALLS (KING SUITE BATH ROOM) XSB	1st – 4x6 STUDS @ 16" o.c., UNO. 2nd – 2x6 STUDS @ 16" o.c. 3rd – 2x6 STUDS @ 16" o.c. 4th – 2x6 STUDS @ 16" o.c. 5th – 2x6 STUDS @ 16" o.c.	
TOWER WALLS	2x4 STUD @ 16" o.c. – MAX HT.= 10'-0" 2x6 STUD @ 16" o.c. – MAX HT.= 15'-0"	
INTERIOR NON-BEARING WALLS	2x4 STUD @ 16" o.c. – MAX HT.= 14'-0" 2x6 STUD @ 16" o.c. – MAX HT.= 20'-0"	

WALL STUD SCHEDULE NOTES:

1. ALL STRUCTURAL STUD LUMBER SHALL BE DOUGLAS FIR LARCH #2 OR BETTER.
2. ALL SILL PLATES IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED.
3. SEE SHEARWALL SCHEDULE ON SN-1 FOR ADDITIONAL WALL FRAMING REQUIREMENTS
4. USE 3x PRESSURE TREATED FOUNDATION SILL PLATE AT SHEAR WALLS.
5. EXTERIOR WALL FRAMING SHALL BE FIRE RETARDANT TREATED WOOD AND SHEATHING. NAILS AT FIRE RETARDANT TREATED EOOD SHALL BE GALVANIZED.

ROOF TRUSS NOTES

TOP CHORD
D.L. = 26.5 PSF*
L.L. = 20.0 PSF

BOTTOM CHORD
D.L. = 5.5 PSF (10.0 PSF @ EXT.)
** L.L. = 10.0 PSF

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WIND
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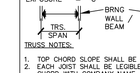
** NOT CONCURRENT w/ TOP CHORD LIVE LOAD.



TRUSS NOTES:

1. TOP CHORD SLOPE SHALL BE AS SPECIFIED BY ARCHITECT
2. EACH JOIST SHALL BE LEGIBLE MARKED ON THE BOTTOM CHORD WITH COMPANY NAME AND TRUSS IDENTIFICATION NUMBER.
3. A CERTIFICATION OF COMPLIANCE IS REQUIRED FOR EVERY TRUSS.
4. SEE ROOF PLAN FOR LOCATION AND WEIGHT OF MECH. EQUIPMENT SUPPORTED ON THE ROOF.
5. PROVIDE FULL DEPTH JOIST BLOCKING AT TRUSSES PERPENDICULAR TO SHEAR WALLS. BLOCKING SHALL EXTEND ENTIRE LENGTH OF SHEAR WALL.
6. PROVIDE BLOCKING AT PARAPET WALL BRACE ATTACHMENT / SUPPORT
7. TRUSS BOTTOM CHORD SHALL BE DESIGNED FOR 250# DEAD LOAD SP/M/P/E EQUIP. ATTACHMENT ALONG ANYWHERE ALONG THE BOTTOM CHORD.
8. SEE SPECIAL GIRDER TRUSS LOAD TABLE (SHEET S-6.0) FOR ADDITIONAL LOADS AT TRUSSES SUPPORTING TOWER WALLS.
9. G.E.T = GABLE END TRUSS, SEE PLANS FOR DRAG LOAD.
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11. "RDT#" INDICATES DRAG TRUSS. DRAG TRUSS SHALL BE DESIGNED/SIZED FOR STRAP ATTACHMENT FOR STRAP SIZES SHOWN ON PLANS. TRUSS SHALL BE DESIGN FOR THE FOLLOWING MINIMUM DRAG CAPACITY:

DTW = PLF SAME AT SHEAR WALL BELOW
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FLOOR/ROOF FRAMING NOTES

1. SEE GENERAL NOTES ON SHEET SN-1 AND SN-2
2. SEE STRUCTURAL STUD WALL SCHEDULE ON THIS DRAWING. SEE ARCHITECTURAL DRAWINGS FOR NON-LOAD BEARING PARTITIONS.
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6. FRAME WALL OPENINGS PER DETAIL 1/SD2
7. ALL NON-BEARING INTERIOR WALLS PER DETAIL 2/SD2
8. TOP PLATE SPLICE PER DETAIL 2/SD2
9. –
10. TYPICAL FLOOR SHEATHING:
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E.N. PLY: 10d @ 6" o.c.
F.N. PLY: 10d @ 12" o.c.
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POST ABREVATION

MARK	SIZE	MARK	SIZE
#P1	SEE NOTE #1	HSS0	HSS 5x5x5/16
P44	4x4 POST	–	–
P46	4x6 POST	–	–
P48	4x8 POST	–	–
P66	6x6 POST	–	–
P68	6x8 POST	–	–

POST NOTES:
1. USE 2-2x STUDS MIN. (# = TOTAL NUMBER OF STUDS WHERE MORE THAN 2-2x STUDS ARE REQUIRED, Ex: 3P1 = 3-2x STUDS), STITCH NAIL EA. STUD w/ 16d @ 12" o.c.

HOLDOWN / VERT. STRAP SCHED.

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V78	MSTC78 (v)	
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V168	CMSTC16 x 8'-0" (v)	
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U11	HDU11 HLDN	
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H-#	INDICATES SPECIAL PATENTED HOLDDOWN SYSTEM. COORDINATE LOCATION WITH FRAMING AT LEVEL ABOVE (** INDICATES SPECIAL ALIGNMENT REQUIRED WITH ABOVE OR BELOW)	SEE SHEETS ATS-1 & ATS-2

LEGEND:

- DETAIL NUMBER / SHEET REFERENCE
- SHEAR WALL TYPE / SHEAR WALL LENGTH
- XX — INDICATES FLOOR JOIST. SEE FLOOR JOIST SCHEDULE, U.N.O.
- XX — INDICATES ROOF TRUSSES @ 24" o.c.
- V-#, U-#, H-# — INDICATES HOLDOWN / VERTICAL STRAP. SEE HOLDOWN SCHEDULE FOR ADDITIONAL INFORMATION.
- ⊠ POST OR TRIMMER STUDS AT BEAMS. USE 2-2x MIN. UNO.
- □ STEEL COLUMN
- WALL BELOW CURRENT FRAMING LEVEL
- WALL ABOVE CURRENT FRAMING LEVEL

HCP ENGINEERING
2466 BEDROCK DR.
CORONA, CA 92881
P: 951.738.0840
F: 951.738.1432
www.hcpengineeringco.com

DESIGN ENGINEERING

HAMPTON INN - SANTA CLARITA
NEWHALL RANCH RD
Santa Clarita, CA 91355

PLAN CHECK NO: BLD19-00927

SEAL
REGISTERED PROFESSIONAL ENGINEER
HITESH C. PATEL
No. C50389
Exp. 6-30-2019
STATE OF CALIFORNIA
10/31/20

TOWER ROOF FRAMING PLAN

REVISION
9/12/19 1 PLAN CHECK #1
12/6/19 2 PLAN CHECK #2

PROJECT No.: 2018-14
PLOT DATE: October 31, 2020
SHEET: **S-7.0**

DESIGN DATA

ROOF DEAD LOADS:

Roofing (Built-up)	= 4.0	psf
Sheathing	= 3.0	psf
Framing	= 3.5	psf
Misc. (Incl. Fire Sprklr) *	= 4.0	psf
Ceiling (2 layer ⅝" Gyp)	= 5.5	psf
Photovoltaic Solar Panels	= 12.0	psf
TOTAL DEAD LOAD	= 32.0	psf

ROOF LIVE LOADS:

TYP. ROOF	= 20.0	psf

FLOOR DEAD LOADS:

1" Gypsum / Lt. Wt. Conc.	= 15.0	psf
Sheathing	= 3.0	psf
Framing	= 4.0	psf
Ceiling (2 layer ⅝" Gyp)	= 5.6	psf
Misc. (Incl. Fire Sprklr)	= 4.4	psf
TOTAL DEAD LOAD	= 32.0	psf

FLOOR LIVE LOADS:

TYP. FLOOR	= 40.0	psf
STORAGE (LIGHT)	= 125.0	psf
CORRIDOR– PRIVATE AREAS	= 40.0	psf
CORRIDOR– PUBLIC AREAS	= 100.0	psf
BALCONY – PRIVATE	= 60.0	psf
BALCONY– PUBLIC AREAS	= 100.0	psf

OTHER LOADS:

INTERIOR PARTIONS	= 10.0	psf
EXTERIOR WALLS	= 15.0	psf
VENEER (1" THK MAX.)	= 8.0	psf
STUCCO CEILING	= 8.0	psf

WIND:

SPEED, V	= 110.0	mph
IMPORTANCE, I	= 1.0	psf
BUILDING CLASSIFICATION	= II	
EXPOSURE	= C	
± GCpi	= ± 0.18	
C & C PRESSURE	= 18.5	psf

SEISMIC:

SPECTRAL ACCELERATION	Ss	= 2.089
	S1	= 0.763
	SDS	= 1.393
	SD1	= 0.661
SITE CLASS		= C
IMPORTANCE		= 1.0
OCCUPANCY		= II

SEISMIC: WOOD FRAMED UPPER LEVELS:

SEISMIC DESIGN CATEGORY	= E
SEISMIC FORCE RESISTING SYSTEM:	= LIGHT FRAMED WOOD SHEAR PANEL
SEISMIC RESPONSE COEFF, Cs	= 0.214
RESPONSE MODIFICATION FACTOR, R	= 6.5
FUNDAMENTAL PERIOD, T	= 0.404
OVERSTRENGHT FACTOR, Ωo	= 3
DEFLECTION AMPLIFICATION, Cd	= 4
ANAYSIS PROCEEDURE	= EQUIV. LATERAL FORCE METHOD

SEISMIC: PORTE COCHERE

SEISMIC DESIGN CATEGORY	= E
SEISMIC FORCE RESISTING SYSTEM:	= STEEL SPECIAL CANTILEVERED COL
SEISMIC RESPONSE COEFF, Cs	= 0.557
RESPONSE MODIFICATION FACTOR, R	= 2.5
FUNDAMENTAL PERIOD, T	= 0.145
OVERSTRENGHT FACTOR, Ωo	= 1.25
DEFLECTION AMPLIFICATION, Cd	= 2.5
ANAYSIS PROCEEDURE	= EQUIV. LATERAL FORCE METHOD

3

SHEAR WALL SCHEDULE [1]

2

TYPE #	SHEATHING MATERIAL	MINIMUM NOMINAL PANEL THICKNESS (in)	MINIMUM FASTENER PENETRATION IN FRAMING (in)	FASTENER TYPE & SIZE	PANEL EDGE FASTENER SPACING (in)	CAPACITY (plf) SEISMIC	CAPACITY (plf) WIND	ANCHOR BOLT SIZE (in)	ANCHOR BOLT SPACING w/ 3x P.T. PLATE (in)	2x SILL PLATE NAILING AT RAISED FLR. & UPPER FLOORS	TOP PLATE ANCHORS (A35 or LTP4) (in)	MINIMUM STUD SIZE AT ADJOINING PANEL EDGES	PERIODIC [3] INSPECTION REQUIRED
9	PLYW'D / OSB	15/32	1 ⅜"	8d	6	260	365	5/8"	46" [2]	16d @ 6" o.c.	20"	2x.	NO
10	PLYW'D / OSB	15/32	1 ⅜"	8d	4	380	532	5/8"	32" [2]	16d @ 6" o.c.	15"	3x.	YES
11	PLYW'D / OSB	15/32	1 ⅜"	8d	3	490	685	5/8"	25" [2]	16d @ 4" o.c.	11"	3x.	YES
12	PLYW'D / OSB	15/32	1 ⅜"	8d	2	640	895	5/8"	19"	SIMPSON SDS ¼"x6" @ 6" o.c.	8"	3x.	YES
13	PLYW'D / OSB	15/32	1 ½"	10d	2	770	1077	5/8"	15"	SIMPSON SDS ¼"x6" @ 4" o.c.	7"	3x.	YES
14	PLYW'D / OSB	19/32	1 ⅜"	10d	2	870	1217	5/8"	14"	SIMPSON SDS ¼"x6" @ 4" o.c.	6"	3x.	YES
15	PLYW'D / OSB BOTH SIDES	15/32	1 ⅜"	8d	3	980	1370	3/4"	20"	SIMPSON SDS ¼"x6" @ 6" o.c.	5"	3x.	YES
16	PLYW'D / OSB BOTH SIDES	15/32	1 ⅜"	8d	2	1280	1790	3/4"	16"	SIMPSON SDS ¼"x6" @ 3" o.c.	8" BOTH SIDES	3x.	YES
17	PLYW'D / OSB BOTH SIDES	15/32	1 ⅜"	8d	2	1540	2156	3/4"	14"	SIMPSON SDS ¼"x6" @ 2 ½" o.c.	7" BOTH SIDES	3x.	YES
18	PLYW'D / OSB BOTH SIDES	19/32	1 ½"	10d	2	1740	2440	3/4"	12"	SIMPSON SDS ¼"x6" @ 6" o.c.	6" BOTH SIDES	3x.	YES

SPECIAL NOTES:

1. SHEAR WALL CAPACITY IS BASED ON ANSI/AF&PA SDPWS–2008, TABLE 4.3A AND ADJUSTED FOR ASD DESIGN PER 4.3.3. AND 4.3.3.2.
2. 2x FOUNDATION SILL PLATES MAY BE USED. ANCHOR BOLT SPACING SHALL BE REDUCED TO ONE–HALF THE SPECIFIED DISTANCE FOR 3x FOUNDATION SILL PLATES.
3. PERIODIC INSPECTION REQUIRED BY A REGISTERED DEPUTY INSPECTOR.
4. EXTERIOR WALL SHEATHING SHALL BE FIRE RETARDANT TREATED SHEATHING. NAILS AT FIRE RETARDANT TREATED WOOD SHALL BE GALVANIZED.

TYPICAL NOTES:

a. WOOD STRUCTURAL PANELS SHALL CONFORM TO THE REQUIREMENTS FOR ITS TYPE IN U.S. PRODUCT VOLUNTARY STANDARD DOC PS–1 AND PS–2
b. PANELS SHALL NOT BE LESS THAN 4'x8', EXCEPT AT BOUNDRIES AND CHANGES IN FRAMING. PANELS SHALL BE INSTALLED VERTICAL. FRAMING MEMBERS OR BLOCKING SHALL BE PROVIDED AT THE EDGES OF PANELS.
c. NAILS LOCATED AT LEAST ⅜" FROM EDGES AND ENDS OF PANELS. MAXIMUM NAIL SPACING OF 6" ON CENTER ALONG PANEL EDGES. MAXIMUM NAIL SPACING OF 12" ON CENTER ALONG INTERMEDIATE FRAMING (FIELD NAILING).
d. ALL FRAMING MEMBERS USED FOR SHEAR WALL CONSTRUCTION SHALL BE 2" NOMINAL OR LARGER. MAXIMUM STUD SPACING SHALL BE 16" ON CENTER.
e. SHEAR WALL BOUNDRY ELEMENTS, SUCH AS END POSTS, SHALL BE PROVIDED AT ALL WALL ENDS. USE (2) 2x MINUMUM, UNLESS NOTED OTHERWISE.
f. SHEAR SHEATHING SHALL NOT BE USED TO SPLICE BOUNDRY ELEMENTS.
g. END POST (STUD OR COLUMN) SHALL BE PROVIDED FULL END BEARING.
h. FRAMING MEMBER SHALL BE DOUGLAS FIR.
i. WHERE PLYWOOD IS APPLIED ON BOTH FACES OF WALL AND NAILING IS LESS THAN 6" O.C., PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS, OR FRAMING SHALL BE 3x OR THICKER, NAILS ON EACH SIDE SHALL BE STAGGERED, SILL PLATES SHALL BE 3x MEMBERS.
j. PLYWOOD JOINT AND SILL PLATE NAILING SHALL BE STAGGERED IN ALL CASES.
k. NAILS SHALL BE COMMON OR GALVANIZED BOX. GALVANIZED NAILS SHALL BE HOT–DIPPED OR TUMBLED.
l. ANCHOR BOLTS SHALL BE INSTALLED A MINIMUM OF 4 3/8" AND A MAXIMUM OF 12" FROM PLATE ENDS. A MINIMUM OF TWO (2) BOLTS PER PLATE. ALL ANCHOR BOLTS SHALL ALSO HAVE A 3"x3"x¼" PLATE WASHER. THE PLATE WASHER SHALL EXTEND TO WITHIN ½" OF THE EDGE OF THE BOTTOM PLATE ON THE SHEATHING SIDE.
m. ALL ANCHOR BOLTS SHALL BE EMBEDED 7" MIN. (WHERE DUAL POUR CONCRETE FOUNDATION SYSTEM IS USED, ANCHOR BOLTS SHALL BE EMBEDED 7" MIN. INTO CONCRETE FIRST POUR.)
n. PROVIDE PLYWOOD END NAILING TO ALL HOLDOWN POSTS.

SHEAR WALL SCHEDULE | A

CONNECTION: CBC TABLE 2304.9.1 *

1. JOIST TO SILL OR GIRDER TOENAIL
2. BRIDGING TO JOIST, TOENAIL EACH END
3. 1"x6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL
4. WIDER THA 1"X6" SUBFLOOR TO EACH JOIST, FACE NAIL
5. 2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL
6. SOLE PLATE TO JOIST OR BLOCKING, TYPICAL FACE NAIL
   SOLE PLATE TO JOIST OR BLOCKING, AT BRACED WALL PANELS
7. TOP PLATE TO STUD, END NAIL
8. STUD TO SOLE PLATE
9. DOUBLED STUDS, FACE NAIL
10. DOUBLED TOP PLATES, TYPICAL FACE NAIL
    DOUBLE TOP PLATES, LAP SPLICE
11. BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOENAIL
12. RIM JOIST TO TOP PLATE, TOENAIL
13. TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL
14. CONTINUOUS HEADER, TWO PIECES
15. CEILING JOIST TO PLATE, TOENAIL
16. CONTINUOUS HEADER TO STUD, TOENAIL
17. CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL
18. CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL
19. RAFTER TO PLATE, TOENAIL
20. 1" BRACE TO EACH STUD AND PLATE, FACE NAIL
21. 1"x8" SHEATHING OR LESS TO EACH BEARING, FACE NAIL
22. WIDER THAN 1"x8" SHEATHING TO EACH BEARING, FACE NAIL
23. BUILT–UP CORNER STUDS
24. BUILT–UP GIRDER AND BEAMS

25. 2" PLANKS

* FASTENERS IN PRESERVATIVE TREATED WOOD ARE TO BE STAINLESS STEEL OR HOT DIPPED ZINC–COATED STEEL (CBC 2304.9.5)

DEFERRED SUBMITTALS:

1. Prefabricated or specialty items and their components, which are indicated by the structural drawings to be designed by others, may be submitted to the architect and/or engineer of record for review as a deferred submittal provided that such submittals is authorized by the building department.
2. All deferred submittals shall include calculations and drawings prepared in accordance with applicable building codes and stamped by a licensed professional engineer.
3. Submittals shall show location and magnitude of loads, size and configuration of members, and compatibility with the primary structural system.
4. Differed submittals required to be submitted to the structural engineer of record shall include but not limited to:

4.1. Premanufactured Roof and Open–Web Floor Trusses: Prior to Rough Framing Inspection: Truss calculations and layout plans will be at the job site with all sheets bearing a California Registered Civil/Structural Engineers wet seal and signature responsible for the design of the trusses; together with the Acceptance Review Stamp by the Building Architect/Engineer and the Building Department's approval stamp.

4.2. Prior to Foundation Inspection City's Building Department Grading Certification Form together with a final geotechnical report indicating site is suitable for proposed construction shall be completed and wet signed and stamped by: Civil Engineer of Record, Geotechnical Engineer and the Grading Contractor with the authorization all work is completed and is in conformance with the approval grading and drainage plans and geotechnical reports and complies with City's Grading Ordinance.

TESTS, INSPECTIONS AND OBSERVATIONS:

1. Continuous inspection by a Registered Deputy Inspector is required for all field welding, high strength bolting, post tensioning, concrete with a strength greater than 2,500 psi, and masonry where noted on the plans and details. The extent of such inspection shall conform to Chapter 17 of the Building Code. An affidavit shall be issued to the Architect/Engineer and the Building Department at the completion of each type of work stating whether the work was in conformance with the approved plans and specifications.

2. In addition to inspections by the Building Department, the following checked items will require Special Inspection in accordance with Chapter 17 of the Building Code.

SPECIAL INSPECTION	REQUIED IF CHECKED	REMARKS
FOUNDATION		
SOIL COMPLIANCE PRIOR TO FOUNDATION INSPECTION BY GEOTECHNICAL ENGINEER	X	OBSERVATION OF SOIL EXCAVATION AND FOUNDATION CONSTRUCTION OPERATION BY GEOTECHNICAL ENGINEER.
CONCRETE:		
HIGH STRENGTH CONCRETE (2,500 PSI OR GREATER)	X	CONCRETE SPECIMEN SAMPLES REBAR & CONTINOUS CONCRETE PLACEMENT
MOMENT RESISTING CONCRETE FRAMES		
PRETENSIONED OR POST–TENSIONED CONCRETE	X	
STEEL:		
SHOP WELDING	X	
FIELD WELDING	X	
HIGH STRENGTH BOLTS	X	
MASONRY:		
PRISM TEST – HIGH STRENTH MASONRY		
REINFORCING AND MASONRY UNIT PLACEMENT	X	PERIODIC SPECIAL INSPECTION
PRIOR TO GROUTING AND GROUTING OPERATIONS	X	
WOOD (Seismic Design Category C, D, E, & F):		PERIODIC SPECIAL INSPECTION REQUIRED FOR NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS WITHIN SEISMIC FORCE RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, DIAPHRAGMS, DRAG STRUTS, BRACES, SHEAR PANELS, AND HOLDOWNS. (SPECIAL INSPECTION NOT REQUIRED WHERE FASTENER SPACING IS MORE THAN 4" o.c.)
SHEAR WALLS & SEISMIC FORCE RESISTING SYSTEM	X	
HOLDOWNS	X	
OTHER:		
SPECIAL GRADING, EXCAVATION & BACKFILL OPERATIONS:		
SHOTCRETE / GUNITE WORK.	X	
PILING, DRILLED PIERS & CASSIONS	X	
EXPANSION ANCHORS / EPOXIED ANCHORS	X	PER CBC SECTION 1705
MECHANICAL AND ELECTRICAL COMPONENTS	X	PERIODIC SPECIAL INSPECTION
SPRAYED FIRE–RESISTANT MATERIAL ON STRUCT. STEEL		
ARCHITECTURAL COMPONENTS: PERIODIC SPECIAL INSPECTION IS REQUIRED DURING THE ERECTION AND FASTENING OF EXTERIOR CLADDING, INTERIOR AND EXTERIOR NONBEARING WALLS AND INTERIOR AND EXTERIOR VENEER IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY D, E or F.	X	EXCEPTIONS: 1. SPECIAL INSPECTION IS NOT REQUIRED ARCHITECTURAL COMPONENTS IN STRUCTURES 30 FEET OR LESS IN HEIGHT. 2. SPECIAL INSPECTION IS NOT REQUIRED FOR CLADDING AND VENEER WEIGHING 5 PSF OR LESS. 3. SPECIAL INSPECTION IS NOT REQUIRED FOR INTERIOR NONBEARING WALLS WEIGHING 15 PSF OR LESS.

STRUCTURAL OBSERVATIONS:

1. Where required by the provisions of CBC Section 1704.6 the owner shall employ the Engineer or Architect Responsible for the structural design to perform Structural Observation.
2. At the conclusion of the work included in the permit, the Structural Observer shall submit to the Building Official a written statement that the visits have been made and identify any reported deficiencies that, to the best of the Structural Observer's knowledge, have been resolved.
3. The Structural Observations shall be performed by the registered design professional responsible for the structural design. The structural observer shall be registered / licensed to practice by the State of California
4. The structural observer shall submit a written statement to Inspection Services a written statement that the site visits have been made and identify any reported deficiencies that to the best of the structural observer's knowledge have bee resolved. The structure will not be in compliance until the registered professional has notified Inspection Services, in writing, that all deficiencies are resolved.

STRUCTURAL OBSERVATION REQ'D (when checked)	REQUIRED IF CHECKED	REMARKS
CONCRETE		PRIOR TO CONCRETE POUR OF EACH ITEM
1. PILE CAGES (PRIOR TO LOWERING THEM)		
2. FOUNDATIONS	X	
3. CONCRETE COLUMN & WALLS		
4. FIRST FLOOR CONCRETE DECK		
5. 2ND LEVEL CONCRETE DECK		
6. HOLDOWN, ANCHOR BOLTS & EMBEDED ITEMS.	X	
MASONRY		AT EACH LEVEL AND PRIOR TO GROUTING OF WALL
1. MASONRY WALLS AND REINFORCING	X	
2. –		
WOOD FRAMING		AT EACH LEVEL PRIOR TO COMMENCING WITH ADDITIONAL LEVEL, PRIOR TO COVERING FRAMING AND PRIOR TO FRAMING FINAL INSPECTION
1. GENERAL FRAMING	X	
2. DIAPHRAGM NAILING	X	
3. SHEAR WALL NAILING	X	
4. HOLDOWN SYSTEM	X	PRIOR TO DRYWALL
STRUCTURAL STEEL:		
1. MOMENT FRAME CONNECTIONS	X	PRIOR TO COVERING OF CONNECTIONS

5. Project Applicant (Owner/Contractor) shall coordinate forms and proceedure with the Building Department.
6. Contractor Responsibility per CBC 1704.4: Each contractor responsible for the construction of a main wind– or seismic–force–resisting system, designated seismic system or a wind– or seismic–resisting component listed in the statement of special inspections (Section 1705) shall submit a written statement of responsibility to the building official and the owner prior to the commencement of work on the system or component. The contractor's statement of resonsibility shall contain the following:
   i) Acknowledgement of awareness of the special requirements contained in the statement of in the statement of special inspections.
   ii) Acknowledgement that control will be exercised to obtain conformance with the construction documents approved by the building official
   iii) Procedure for exercising control within the contractor's organization, the method and frequency of reporting and the distribution of the reports
   iv) Identification and qualifications of the person(s) exercising such control and their position(s) in the organization.

CITY OF SANTA CLARITA
BUILDING & SAFETY
APPROVED
Under SCMC Titles 18, 19, 20, 21, 24, 25
Nov 3, 2020
R. Abdel-Messih
BLD19-00927
The approved plans must be available at the construction site at all times. Changes or alterations to approved plans shall not be made without written permission from the City of Santa Clarita Building & Safety Division. The approval of these plans shall not be construed to permit or approve any violation of the applicable codes, ordinances, or other laws.

WOOD FRAME:

1. All structural lumber shall be Douglas Fir of the following grades, conforming to the Standard Grading Rules for West Coast Lumber, No. 16, unless noted otherwise.

Rafters, joists	No. 2	1250 F
Beams and stringers	No. 1	1300 F
Posts and timbers	No. 1	1200 F
2x4 joists and rafters	No. 2	1250 F
Studs, blocking, plates	No. 2	1250 F

2. All wood bearing on concrete or masonry or within 8" of earth shall be pressure treated Douglas Fir.
3. Each sheet of plywood shall be identified by a registered stamp or brand of the American Plywood Association.
4. Roof sheathing shall be plywood (CDX) or OSB. Use "EXTERIOR" OR "EXPOSURE 1" identified plywood/OSB where exposed to weather. Plywood/OSB for floor sheathing shall be APA RATED SHEATHING or STURD–I–FLOOR Plywood/OSB. All plywood/OSB shall be glued with exterior type glue. All plywood/OSB shall conform to U.S. Product Standard PS 1–95 or PS2 Performance Standard for Wood–based Structural–Use Panels.
5. Nails shall be common. Nailing shall be per Table 2304.9.1 of the California Building Code unless noted otherwise on the plans or details.
6. All metal connectors shall be Simpson Strong–Tie connectors. The nails for these connectors shall be joist hanger nails as manufactured or specified by Simpson Co.. All metal connectors are design for full capacity nailing per Simpson Strong–Tie catalog (ALL NAIL HOLES SHALL BE USED)
7. Provide 1x6 let–in bracing (at approx. 45 degrees) every 25'–0" maximum in stud walls not sheathed. Bracing shall run continuous from sill plate to top plate. Nail with 2–8d per stud and 3–8d each end to plates.
8. Provide fire stops at all intersections of stud walls at floor, ceiling and roof. Fire stops shall be 2x nominal thickness of wood and shall be the full width of the enclosed space. Place fire stops at a maximum spacing of 10'–0" in the vertical direction. Provide 2x fire stops in all furred spaces, vertical and horizontal, and at a maximum spacing of 10'–0" in each direction and at the same lines as fire stops in adjacent stud walls.
9. Top plates of all stud walls shall be 2 pieces the same size as studs. Splices to lap 4'–0" minimum and be nailed per the details.
10. Bolt holes in wood shall be 1/32" to 1/16" larger than the nominal bolt diameter. All bolts shall have standard cut washers under the head and nut unless noted otherwise.
11. All bolts shall be retightened prior to the application of sheathing, plaster, etc.
12. Structural members shall not be cut for pipes, etc. unless specifically detailed.
13. Provide 2x solid blocking between joists and rafters at all supports. Blocking shall be one piece and the full depth of the joist or rafter.
14. Cross bridging or solid blocking shall be provided at 8'–0" o.c. maximum for all floor joists more than 12" deep and at 10'–0" o.c. for all rafters more than 8" deep.
15. Provide double joists under partitions which are parallel to the floor joists.
16. Fasteners in preservative–treated wood (Anchor Bolt, Nails, Screws...etc) are to be approved silicon bronze, copper, stainless steel or hot–dipped zinc coated steel (CBC.2304.9.5)
17. All conventional lumber shall have maximum 19% moisture content.

TRUS JOIST PRODUCTS:

1. Manufacturer shall supply to the Architect/Engineer and the Building Department calculations and shop drawings for approval prior to fabrication. All calculations and shop drawings shall be signed by a registered Professional Engineer. It shall be the responsibility of the manufacturer to obtain Building Department approval of calculations and shop drawings prior to fabrication.
2. Trusses shall be designed in accordance to the latest local Building Code for all loads imposed, including lateral loads and mechanical equipment loads.
3. Trusses shall be manufactured with steel webs, true pin connections and structural wood chords.
4. Trusses and parts shall have product acceptance from ICC ES ER–PFC4979 and ES PFC5676P.
5. Cross bridging and/or bracing shall be provided and detailed as required to adequately brace all beams. Blocking and/or bridging should be installed as erection proceeds and temporary bracing installed to maintain alignment and prevent lateral movement until the sheathing is fastened to the top flange.
6. Temporary removing web members and drilling or cutting of chords is not permitted.
7. Sheathing shall be securely fastened to the top chord. The nailing pattern should be staggered to avoid splitting and to assure nailing into each chord member.

DESIGN PROPERTIES (ICC-ES-ESR-1387)

GRADE	G SHEAR MODULUS OF ELASTICITY (PSI)	E MODULUS OF ELASTICITY (PSI)	Fb FLEXTURAL STRESS (PSI)	Ft TENSION STRESS (PSI)	Fcpn COMP. PERPEN. TO GRAIN (PSI)	Fcpr COMP. PARALLEL. TO GRAIN (PSI)	Fv HORIZ. SHEAR PARALLEL TO GRAIN (PSI)	SG EQUIV. SPECIFIC GRAVITY
LSL (TIMBERSTRAND)	81,250	1.3E6	1,700	1,075	680	1,400	400	0.50
LVL (MICROLAM)	118,750	1.9E6	2,600	1,555	750	2,050	285	0.50
PSL (PARALLAM)	125,000	2.0E6	2,900	2,025	750	2,900	290	0.50

WOOD TRUSSES:

1. Manufacturer shall supply to the Architect/Engineer and the Building Department calculations and shop drawings for approval prior to fabrication. All calculations and shop drawings shall be signed by a California registered Professional Engineer. It shall be the responsibility of the manufacturer to obtain Building Department approval of calculations and shop drawings prior to fabrication.
2. Trusses shall be designed in accordance with the latest local Building Code for all loads imposed, including lateral loads and mechanical equipment loads.
3. All connectors shall be ICC approved and of adequate strength to resist stresses due to the loading involved.
4. Dead Load deflection shall be limited to L/240.
5. Cross bridging and/or bracing shall be provided and detailed as required to adequately brace all trusses.
6. Each truss shall be legibly branded, marked or otherwise have permanently affixed thereto the following information located within 2 feet of the center of the span on the face of the bottom chord:
   a) Identity of the company manufacturing the truss.
   b) The design load.
   c) The spacing of the trusses.
   d) Truss Identification Number corresponding with the Truss Layout Plan
7. DEFERRED TRUSS PLANS: Truss design drawings sealed and wet–signed by a Registered Engineer shall be submitted to the Engineer Of Record (EOR) for review and approval. The Engineer Of Record will than place a shop drawing approval stamp and signature on all associated layout sheets. Contractor shall be responsible for submission to the Building Department for record purposes prior to the roof framing inspection.

STRUCTURAL STEEL:

1. All fabrication and erection shall conform to the latest edition of the AISC Manual of Steel Construction.
2. All welding shall comply with A.W.S. Specifications and is to be done by Certified Welders approved by the Department of Building and Safety. All welding will be done by electronic arc process using ASTM E70XX electrodes and shall be performed with approved electrodes as required per the A.I.S.C. specification. Welds are designed at full stress and must be done in the shop of a licensed fabricator. Special contiuous deputy inspection is required for all field welding.
3. All structural steel shall conform to ASTM A992, Grade 50.
4. Anchor bolts and unfinished bolts shall conform to ASTM A307.
5. All pipe shall conform to ASTM A53, Grade B (35 ksi).
6. All tube shall conform to ASTM A500, Grade B (46 ksi).
7. Plates and miscellaneous iron shall conform to ASTM A36, Unless Otherwise Noted.
8. All structural steel shall be fabricated and erected by a licensed fabricator approved by the Department of Building and Safety.
9. All grout under steel bearing plates shall be solid dry pack or non–shrink grout placed as directed by the manufacturer.
10. High–strength bolts shall conform to the provisions of the "Specification For Structural Joints Using ASTM A325 or A490 Bolts", latest edition, as approved by the Research Council on Riveted and Bolted Structural Joints, and the provisions of U.B.C. Standard No. 27–7.
11. The assembly surface, including those adjacent to the washer, shall be free of mill scale, oil, paint or other coatings.
12. All H.S.B. shall be tightened to a bolt tension not less than that given in Table 1.23.5 of the AISC Manual. Tightening shall be done by the turn–of–nut method, by a direct tension indicator, or by properly calibrated wrenches. Provide hardened washers under the nut or bolt head, whichever is the element turned in tightening.
13. Weld at "Demand Critical Connection" of moment resisting frame beams to columns shall be in accordance with the AISC Seismic Design Manual and ANSI/AISC 341–5 Sections 7.3, 7.3a and 7.3b. Weld filler material must have a Charpy V–Notch toughness of 20ft–lb at –20°F.

GLUED LAMINATED LUMBER:

1. All fabrication and workmanship shall conform to the current edition of the Standard Specifications for Structural Glued Laminated Douglas Fir (Coast Region) Lumber by the West Coast Lumbermen's Association. ATC 117–82, product Standard 56–73, and the current edition of Timber Construction. A certificate of inspection for each Glu–Lam beam from an approved testing agency shall be submitted to and approved by the local Building Department and by the Engineer prior to erection.
2. All glued laminated members shall be Douglas Fir, Combination 24F with waterproof resorcinol or phenol resorcinol glue conforming to the Federal Specifications MIL–A–397–B. Core laminations may be hem fir. Use combinations 24F–V4 or 24F–V5 for simple supported beams and combination 24F–V8 or 24F–V10 for cantilevered beams.
   Fb = 2,400 psi Fv = 215 psi E = 1,800,000 psi
3. Finish of the members shall be Industrial Appearance Grade in conformance with the Standard Appearance Grades of the A.I.T.C.

GENERAL NOTES: 3

1. All construction and workmanship shall conform to the 2019 California Building Code.
2. These notes shall be used in conjunction with the plans and any discrepancies shall be bought to the attention of the Architect/Engineer.
3. Contractor must check all dimensions, framing conditions, and site conditions before starting work. Architect/Engineer shall be notified immediately of any discrepancies or possible deficiencies.
4. Conditions not specifically shown shall be constructed similar to the details for the respective materials.
5. The drawings and specifications represent the finished structure. All bracing, temporary supports, shoring, etc. is the sole responsibility of the Contractor. Observation visits to the site by the Architect/Engineer do not include inspection of construction procedures. The Contractor is solely responsible for all construction methods and for safety conditions at the worksite. These visits shall not be construed as continuous and detailed inspections.
6. Design, materials, equipment, and products other than those described below or indicated on the drawings may be considered for use, provided prior written approval is obtained from the Owner, Architect/Engineer, and the applicable governing code authority. Approval of submittal and shop drawings shall not be considered as written approval unless a written request specifying the substitution is provided and accepted by Architect/Engineer.
7. All conditions noted as existing are based on the information currently available at the time of preparation of these drawings. The Contractor is to verify all conditions before starting work. Should conditions arise which are different from those shown on the drawings, the Architect/Engineer shall be notified immediately and additional drawings based on more accurate information will be prepared.
8. NOTICE TO THE APPLICANT/OWNER/OWNER'S AGENT/ARCHITECT or ENGINEER OF RECORD: By using this permitted construction drawings for construction/installation of the work specified herein, you agree to comply with the requirements of City Building Department for special inspections, structural observations, construction material testing and off–site fabrication of building components, contained in the statement of special inspections and, as required by the California construction codes.
9. NOTICE TO THE CONTRACTOR/BUILDER/INSTALLER/SUB–CONTRACTOR/OWNER–BUILDER: By using this permitted construction drawings for construction/installation of the work specified herein, you acknowledge and are aware of the requirements contained in the statement of special inspections. You agree to comply with the requirements of the City Building Department for special inspections, structural observations, construction material testing and off–site fabrication of building components, contained in the statement of special inspections and as required by the California construction codes.
10. The special inspector must be certified by the City Building Department in the category of work required to have special inspection.
11. The construction materials testing laboratory must be approved by the City Building Department for the testing of materials, systems, components and, equipments.
12. Fabrication must be registered and approved by the City Building Department for the fabrication of members and assemblies on the premises of the fabricator's shop.
13. Fabricators shall submit an 'Application to perform Off–Site Fabrication' to the Inspection Services Division for approval prior to commencement of fabrication.
14. Fabricator shall submit a 'Certificate of Compliance for Off–Site Fabrication to the Inspection Services Division prior to erection of fabricated items and assemblies.

FOUNDATIONS:

1. All disturbed surfacial soils are to be excavated and replaced as compacted fill per the requirements of the soils report.
2. All excavations, filling, backfilling, recompaction and building pad shall be inspected and approved by a Soils Engineer prior to the placement of any concrete or reinforcing. The contractor shall be responsible for all shoring necessary to support cut and/or fill sections during excavation and for forming and placement of concrete.
3. Footings are to be carried a minimum of 24" (Unless Noted Otherwise)into firm undisturbed natural soils or approved compacted fill.
4. Design bearing pressure is 3,000 psf (DL+LL) with 33 % increase for seismic or wind loading.
5. Relatively non–expansive fill should be used in backfilling behind walls. All walls shall be adequately shored during backfilling operation.
6. Fasteners in preservative–treated wood (Anchor Bolt, Nails, Screws...etc) are to be approved silicon bronze, copper, stainless steel or hot–dipped zinc coated steel (CBC.2304.9.5)
7. All excavation, grading, compaction, etc. shall be accomplished and performed in accordance with the Soils Report. The Soils Report is hereby made a part of these General Notes and all recommendations contained therein shall be considered as minimums.
   Soils Report prepared by:
   1. R.T. FRANKLIN & ASSOCIATES. 2602 HUNTINGTON LANE, SUITE A. SANTA CLARITA, CA 91355 JOB NO. 2006–309–021, DATED: MARCH 19, 2018
   2. INTERIM COMPACTION REPORT, PROPOSED HOTEL BUILDING PAD, 28700 NEWHALL RANCH ROAD, APN 286–035–007, VALENCIA, CITY OF SANTA CLARITA, CALIFORNIA BY SOUTHWEST GEOTECHNICAL, INC., 21704 WEST GOLED TRIANGLE ROAD, SUITE 425, SANTA CLARITA, CA 91350, DATED JANUARY 11, 2019 SGI #1812330

REINFORCING STEEL

1. Reinforcing steel shall conform to ASTM A615, Grade 40 for sizes #3 and #4 and Grade 60 for sizes #5 and larger.
2. Welded fabric (mesh) shall conform to the latest revised ASTM A185. Smooth wire fabric shall conform to ASTM A85, yield strength 60 ksi.
3. Welding of reinforcing steel shall conform to AWS D12–1 using proper low hydrogen electrodes. All bars to be welded shall conform to ASTM A706.
4. All bars in masonry shall be lapped a minimum of 40 bar diameters (2'–0" min.) at all splices unless noted otherwise.
5. All bars in concrete shall be lapped a minimum of 36 bar diameters (2'–0" min.) at all splices unless noted otherwise.
6. Splices of horizontal rebar in walls and footings shall be staggered 4'–0" minimum.
7. Dowels for walls and columns shall be the same size and spacing as the wall/column reinforcing unless noted otherwise.
8. All bending of reinforcing steel shall conform to the latest edition of the California Building Code.
9. All concrete embeded items, anchor bolts, holdowns, reinforcing steel etc. shall be secured in place prior to inspection.

CONCRETE:

1. All concrete shall attain a minimum compressive strength of 3,000 psi at 28 days, unless noted otherwise. (W/C Ratio: 0.45)
2. Aggregates shall be natural sand and rock conforming to ASTM C33.
3. Cement shall be Portland Cement conforming to ASTM C–150, Type V.
4. The following minimum clear distances between reinforcing steel and face of concrete shall be maintained unless noted otherwise:
   Slab on grade .... Center of Slab
   Concrete below grade, formed .... 2"
   Concrete below grade, unformed (pour against earth) .... 3"
   Concrete exposed to weather .... 1 ½"
5. Pipes may pass through structural concrete in sleeves, but shall not be embedded therein. Pipes or ducts exceeding one–third the slab or wall thickness shall not be placed in the structural concrete unless specifically detailed.
6. Provide 3/4" chamfer at all exposed corners.
7. Refer to architectural drawings for reveals, areas of textured concrete or special finishes, items required to be cast into the concrete, curbs and slab depressions.

MASONRY:

1. All masonry shall be reinforced grouted masonry. Grout soil all cells which contain rebar, bolts, etc. Grout solid all cells below grade.
2. Mortar shall be Type "S" for walls above grade and Type "M" for retaining walls. Mortars shall be mixed in the proportion of 1 part Portland Cement to ½ to ¼ parts lime putty to 2 ¼ to 3 times the sum of the cement plus lime putty parts of sand.
3. Grout shall attain a minimum compressive strength of 2000 psi and shall be mixed in proportion of 1 part Portland Cement to 1/10 part lime putty to 2 to 3 parts sand to a maximum of 2 parts gravel.
4. Aggregates for mortar and grout shall be natural sand and rock conforming to ASTM C–144 (mortar) and C–404 (grout).
5. Cement shall be Portland Cement conforming to ASTM C–150, Type I or II, low alkali.
6. All concrete block shall conform to ASTM C90, Grade N–1.
7. All brick shall conform to ASTM C62, Grade MW.
8. All reinforcement, bolts, etc. shall have a minimum grout coverage of ¾". All brick shall have a minimum of 2 ½" grout space.
9. See the architectural drawings for type of units, laying pattern and joint details. Unless specifically shown otherwise, all concrete block and brick shall be laid in running bond.

REVISIONS:

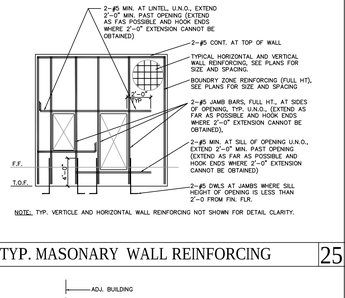
2	PLAN CHK #2 12/6/19
3	10/30/20 CBC2016 UPDATE

HCP ENGINEERING
1307 West Sixth Street, Suite 211
Corona, CA 92882-8605
Ph: (909) 738-0840
Fax: (909) 738-1432

GENERAL NOTES & TYPICAL DETAILS

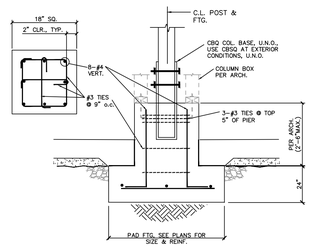
REGISTERED PROFESSIONAL ENGINEER
HITESH C. PATEL
No. C50389
Exp. 6-30-2019
CIVIL
STATE OF CALIFORNIA
10/31/20

PLOT DATE: 10/31/2020
SCALE:
DRAWN BY:
JOB NO. 2018–14
SHEET NO. SN-1

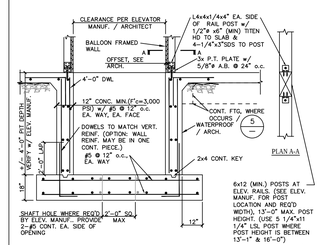


NOTE: TYP. VERTICLE AND HORIZONTAL WALL REINFORCING NOT SHOWN FOR DETAIL CLARITY.

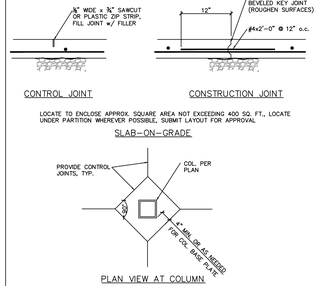
31 TYP. MASONARY WALL REINFORCING



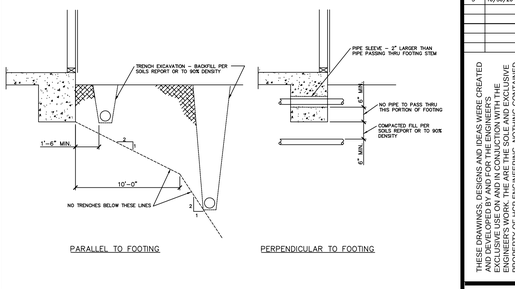
25 POST ON CONCRETE PEDESTAL



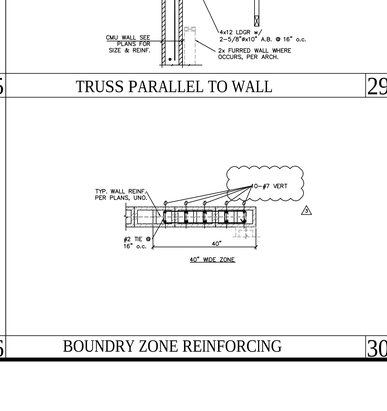
19 ELEVATOR PIT



13 TYP. SLAB CONST. & CONTROL JOINT



1 TRENCHING DETAIL



32

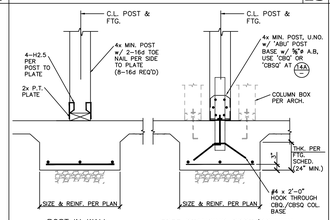
33

27

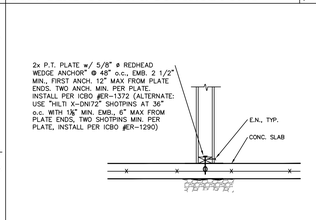
BAR SIZE	F'c <= 3000 p.s.i.		F'c = 4000 p.s.i.		MASONRY WALL (HORIZONTAL & VERTICAL REINF)
	TOP BARS HORIZONTAL	BOTTOM BARS	TOP BARS HORIZONTAL	BOTTOM BARS	
#3	18"	16"	16"	16"	27"
#4	28"	22"	24"	20"	36"
#5	42"	32"	36"	28"	45"
#6	56"	44"	50"	38"	54"
#7	82"	64"	72"	54"	63"
#8	94"	72"	82"	62"	72"
#9	106"	82"	92"	70"	81"
#10	118"	92"	102"	80"	
#11	132"	102"	114"	88"	

MINIMUM COVER SHALL BE GREATER OF 3/4" OR BAR DIAMETER.
MINIMUM BAR SPACING (NOT LAPPING BARS) SHALL BE 2 BAR DIAMETERS CLEAR.

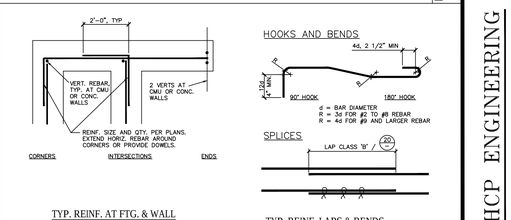
20 MINIMUM REINFORCING LAP LENGHTS



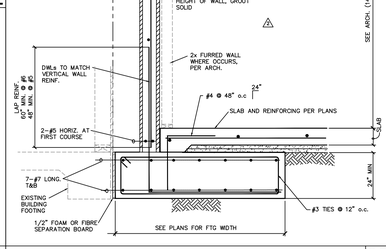
14 COLUMN BASE



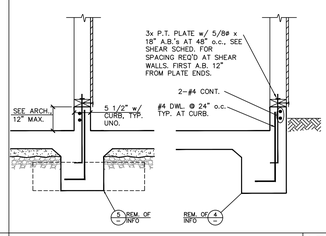
8 TYP. INTERIOR NON-BEARING WALL ANCHORAGE



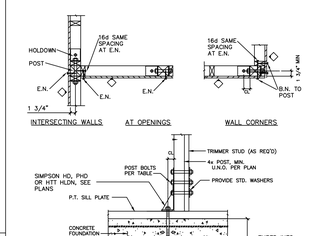
2 TYPICAL REINFORCING REQUIREMENTS



27 EQUIPMENT PAD



15 FOOTING WITH CURB

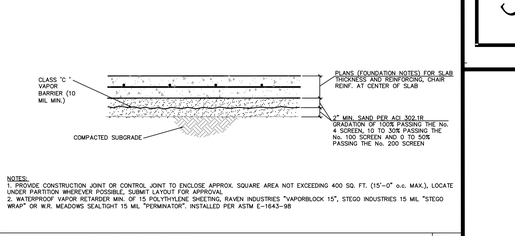


ELEVATION

FASTENERS									
SIMPSON MODEL NO.	MINIMUM POST, UNO	DIM FOR BOLT PLACEMENT "CL"	HOLDOWN ANCHOR DIA	APPLICABLE SIMPSON SSTB ANCHOR		STUD BOLTS		CAPACITY (LBS)
				MONO POUR	DUAL POUR	QTY	DIA	
HTT4	4x4	1 3/8	5/8	SSTB16	SSTB20	18	16d	2550
HTT5	4x4	1 3/8	5/8	SSTB24	–	26	10d	3740
HDQ8	4x4	1 1/4	7/8	SSTB28	SSTB34	20	SDS3	7630
HHDQ11	4x6	1 1/2	1 **	SB1x30	SB1x30	24	SDS2.5	11410
HHDQ14	4x6	1 1/2	1 **	SB1x30	SB1x30	30	SDS2.5	13015
HDU2	4x4	1 1/4	5/8	SSTB16	SSTB20	6	SDS2.5	2550
HDU4	4x4	1 1/4	5/8	SSTB20	SSTB24	10	SDS2.5	3145
HDU5	4x4	1 1/4	5/8	SSTB24	SSTB34	14	SDS2.5	3740
HDU8	4x6 or 6x6	1 1/4	7/8	SSTB28	SSTB34	20	SDS2.5	8315
HDU11	4x6 or 6x6	1 1/4	1	SB1x30	SB1x30	30	SDS2.5	9535
HDU14	4x6 or 6x6	1 9/16	1	SB1x30	SB1x30	36	SDS2.5	14390
HD12	4x6 or 6x6	2 1/8	1 1/8 **	SB1x30	SB1x30	4	1" ø	15510
HD19	4x6 or 6x6	2 1/8	1 1/4 **	–	–	5	1" ø	19070

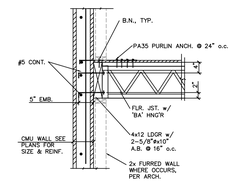
** PROVIDE ANCHOR BOLT OR ALL THREAD ANCHOR WITH 3/8"x3" SQ. PLATE WITH WASHER AND DOUBLE NUT. SEE DETAIL FOR REQUIRED EMBEDMENT AT 1 1/8"ø & 1 1/4"ø A.B.

10 HD HOLDOWN DETAIL

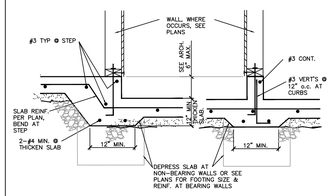


NOTES:
1. PROVIDE CONSTRUCTION JOINT OR CONTROL JOINT TO ENCLOSE APPROX. SQUARE AREA NOT EXCEEDING 400 SQ. FT. (15'–0" o.c. MAX.), LOCATE UNDER PARTITION WHEREVER POSSIBLE. SUBMIT LAYOUT FOR APPROVAL
2. WATERPROOF VAPOR RETARDER MIN. OF 15 POLYTHYLENE SHEETING, RAVEN INDUSTRIES "VAPORBLOCK 15", STEGO INDUSTRIES 15 MIL "STEGO WRAP" OR W.R. MEADOWS SEALTIGHT 15 MIL "PERMINATOR", INSTALLED PER ASTM E–1643–98

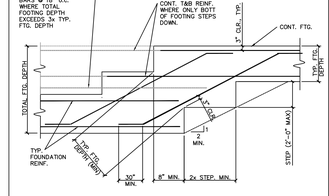
3 TYPICAL SLAB-ON-GRADE



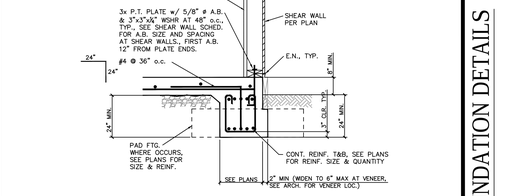
28 JOIST PERPEND. TO WALL



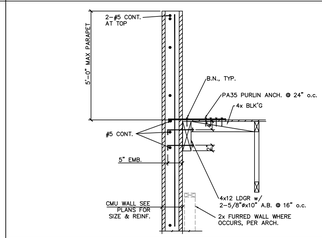
22 STEP SLAB, DEPRESSION & CURBS



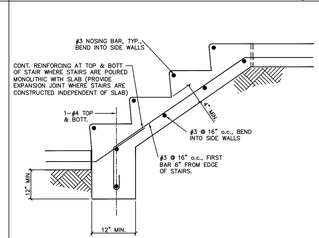
16 STEP FOOTING



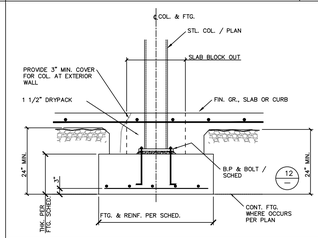
4 TYP. EXTERIOR FOOTING



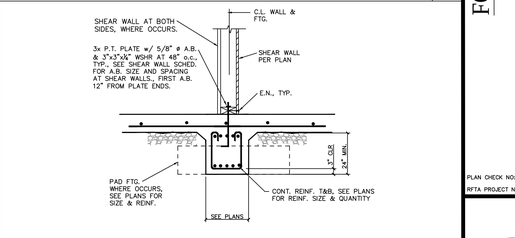
29 TRUSS PARALLEL TO WALL



17 CONCRETE STEPS

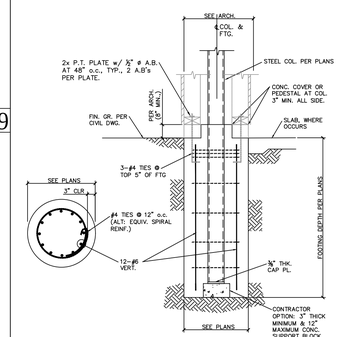


11 FOOTING AT STEEL COL.

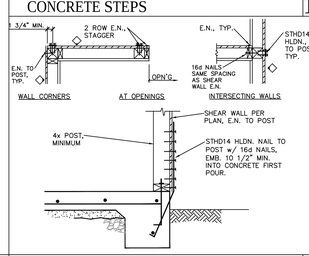


5 TYP. SHEAR WALL FOOTING

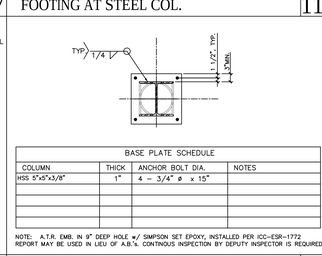
30 BOUNDRY ZONE REINFORCING



24 POLE FOOTING



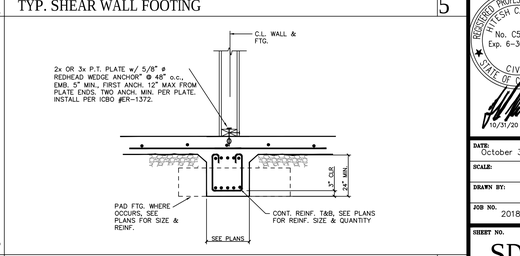
18 STHD HOLDOWN



BASE PLATE SCHEDULE			
COLUMN	THICK	ANCHOR BOLT DIA.	NOTES
HSS 5"x5"x3/8"	1"	4 – 3/4" ø x 15"	

NOTE: A.T.R. EMB. IN 9" DEEP HOLE w/ SIMPSON SET EPOXY, INSTALLED PER ICC–ESR–1772 REPORT MAY BE USED IN LIEU OF A.B.'s. CONTINOUS INSPECTION BY DEPUTY INSPECTOR IS REQUIRED

12 BASE PLATE FOR STEEL COL. ⚠



6 TYP. INTERIOR BEARING (NON-SHEAR) WALL FOOTING

CITY OF SANTA CLARITA
BUILDING & SAFETY

APPROVED
Under SCMC Titles
18, 19, 20, 21, 24, 25
Nov 3, 2020
R. Abdel-Messih

BLD19-00927

The approved plans must be available at the construction site at all times. Changes or alterations to approved plans shall not be made without written permission from the City of Santa Clarita Building & Safety Division. The approval of these plans shall not be construed to permit or approve any violation of the applicable codes, ordinances, or other laws.

REVISIONS:		
1	9/12/19,	PLAN CHK. #1
2	12/8/19,	PLAN CHK. #2
3	10/30/20	CBC2016 UPDATE

HCP ENGINEERING
2466 Redrock Dr.
Norco, California 92860
Ph: (951) 738–0840
Fax: (951) 738–1432

FOUNDATION DETAILS

PLAN CHECK NO: BLD–17–00736
RFTA PROJECT NO.: 750

REGISTERED PROFESSIONAL ENGINEER
HITESH C. PATEL
No. C50389
Exp. 6–30–2019
CIVIL
STATE OF CALIFORNIA
10/31/20

DATE: October 31, 2020
SCALE:
DRAWN BY:
JOB NO. 2018–14
SHEET NO. SD1

PLYWOOD SHTG TO CONTINUE THROUGH GAP
2x PLATE w/ 16d @ 12" o.c., SEE SHEAR WALL SCHED. FOR SILL PLATE NAILING AT SHEAR WALLS
B.N. FUR. SHTG TO BLK'G & PLATES
OPTION: SPLICE PLYWOOD AT BEAM OR PROVIDE FILLER
TRS. PER TRS. MANUF.
CONTINUOUS 2 LAYERS 5/8" TYPE-X DRYWALL BOTH SIDES OF WALL
SHEAR WALL, WHERE OCCURS, E.N. TO PLATES
SEE ARCH. DRAWING FOR REMAINDER OF WALL & FLOOR-CEILING ASSEMBLY

2-HOUR FIRE SEPARATION (JST. PARALLEL TO WALL) 37

PLYWOOD SHTG TO CONTINUE THROUGH GAP
'DHU' HNGR o/ DRYWALL (INSTALLED FLUSH w/ TOP OF WALL)
2x BLK'G w/ 16d@3" o.c. TO PLATE
2x PLATE w/ 16d @ 12" o.c., SEE SHEAR WALL SCHED. FOR SILL PLATE NAILING AT SHEAR WALLS
B.N. FUR. SHTG TO BLK'G & PLATES
OPTION: SPLICE PLYWOOD AT BEAM OR PROVIDE FILLER
2x PLATE (SPLICE AT CENTER OF STUD)
"SSP" STUD PLATE TIE AT BACK SIDE TO EACH STUD
BOTTOM CHORD BEARING TRUSS w/ TRS. HNGR
CONTINUOUS 2 LAYERS 5/8" TYPE-X DRYWALL BOTH SIDES OF WALL
SHEAR WALL, WHERE OCCURS, E.N. TO PLATES
SEE ARCH. DRAWING FOR REMAINDER OF WALL & FLOOR-CEILING ASSEMBLY

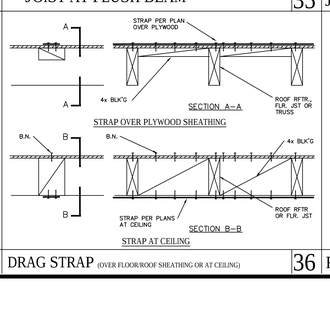
2-HOUR FIRE SEPARATION (CHANGE JST. DIR) 38

EXTERIOR
STAIR
EXTERIOR
RIM JST., SAME WIDTH AS STUDS & DEPTH AS FLOOR JST, U.N.O., SPLICE AT CORNERS w/ 2-ST6236 BENT.
RIM JST, MITRE AT CORNER, TYP. (MIN)
2-ST6236 BENT AT CORNERS
CONT. DRYWALL PER ARCH.
2x OR 1 ¾" PLYWD TREADS
RIM JST AT CORNER
4x BLK'G
4x BLK'G
2x LDG'R w/ 4-1/4" SDS or 4 1/2" SCREWS PER STUD & 16d @ 6" o.c. TO BLK'G
JST. @ MID-LEVEL LANDING w/ 'HUS' HNGR
STRINGERS w/ 4-1/4" SDS or 4 1/2" SCREWS PER STUD & 16d @ 6" o.c. TO BLK'G
FRAMING AT MID-LEVEL LANDING
FRAMING AT STRINGERS

STAIRS @ MIDDLE LANDING 39

RAIL BRKT, PLATE & LAG SCREWS PER ELEV. MANUF.
8" MIN.
PROVIDE 2x12 (MIN.) @ 12" o.c. TO TRS. JOIST CENTERED w/ ELEV. RAIL POST.
PLAN A-A
SHAFT, SEE ELEV. MANUF.
SHEAR WALL PER PLAN. SPLICE AT RIM JST. EN. TO ALL PLATES AND BEAMS (TYP. 10 MIN. IF NON NOTED)
1 1/4"x4" SDS @ 8" o.c. MAX. TOP & BOTT PLATE TO RIM JST. AT ELEV. WALL
RAIL BRACKET, WHERE OCCURS, BY ELEV. MANUF.
6x (MIN) RIM JOIST SAME DEPTH AS FLR JST (6x10 MIN) ALL SIDES OF ELEV. SHAFT. 6x12 MIN. POSTS AT ELEV. RAILS. SEE ELEV. MANUF. FOR POST LOCATION (USE 5 1/4"x11 1/4" LSL POST WHERE POST HEIGHT EXCEEDS 14'-0")
L4x4x1/4x0'-5" EA. SIDE OF RAIL POST w/ 4-1/4"SDSx6" TO PLATE & 4-1/4"x3"SDS TO POST (TOP & BOTT)
B.N. TO BLK'G
2x SOLID BLK'G @ 24" o.c. ENTIRE LENGTH OF ELEV. SHAFT MINIMUM 3 BAYS
B-B
2x BLK'G
'DHU' HNGR o/ DRYWALL (INSTALLED FLUSH w/ TOP OF WALL)
JOIST PERPEND. TO SHAFT WALL
JOIST PARALLEL TO SHAFT WALL

FRAMING AT 2 HR. ELEV. SHAFT (OPEN WEB JOISTS) 41



DRAG STRAP (OVER FLOOR/ROOF SHEATHING OR AT CEILING) 36

40

'DHU' HNGR o/ DRYWALL (INSTALLED FLUSH w/ TOP OF WALL)
2x BLK'G w/ 16d@3" o.c. TO PLATE
POST
HLDN. TYP., SEE PLANS FOR SIZE & SIMPSON CATALOG FOR NO. & SIZE OF BOLTS POST & THREADED ROD BETWEEN HLDN's
6 1/2" MIN.
SOLID BLK'G UNDER POSTS
POST
"HDU" HOLDOWNS

E.N. PLYWOOD SHEAR WALL TO POST
VERTICLE (V) STRAP PER PLAN, U.N.O.
"END LENGTH" PER SIMPSON STRONG-TIE MIN. w/ 16d @ 1½" o.c. FOR "CS" & "CMST" STRAPS
"END LENGTH" PER SIMPSON STRONG-TIE MIN. w/ 16d @ 1½" o.c. FOR "CS" & "CMST" STRAPS
BEND LONG STRAPS AROUND BEAM, WHERE OCCURS
VERTICAL STRAP

HOLDOWN TO WALL BELOW 32

26

STRAP PER PLAN (CS14x8'-0, MIN. U.N.O.)
10d @ 3" o.c. TYP. TO BLK'G
4x BLK'G
TRUSS, TYP.
24" @ "CS" STRAPS
36" @ "CMST" STRAP
STRAP LENGTH PER PLANS

DRAG (JOIST PERPENDICULAR STRAP) 25

1 1/8" PLYWOOD THREADS (MIN)
FUR. JST PER PLAN
FL. BM.
SIMPSON 'MIT.81/14' HNGR
STRINGERS PER PLAN (USE 1-3/4"x14" 1.5E TIMBERSTRAND LSL @ 16" o.c. U.N.O.)
SEE ARCH PLANS FOR NUMBER OF TREADS...ETC.
FLR. JST. PER PLAN
FL. BM.

TYP. FRAMING AT STAIRS 27

STRAP PER PLAN (MSTC40, MIN. U.N.O.)
16d @ 3" o.c. TO TRUSS (STAGGER WHERE POSSIBLE)
BOTT. OF PLY'W SHT'G
2x6 (MIN) SCAB TO TRUSS
2x STRAP LENGHT ATTACHED TO SCAB (48" MIN)
DRAG JOIST OR FL. BEAM PER PLAN (NOTE: WHERE BEAM OR SOLID FLOOR JOIST OCCURS, OMIT SCAB)

DRAG AT DRAG BEAM / JOIST 28

REM OF INFO @ SIM.
B.N.
SHEAR WALL E.N.
TRS. PER TRS. MANUF.
FLR. JST. AT CORRIDOR w/ 'LUS' HNGR
FLUSH BM. WHERE OCCURS
2x LEDGER w/ 16d @ 12" o.c. TO PLATES AND 4-16d PER STUD
CONT. SHEAR WALL, WHERE OCCURS
DBL. SIDE SHEAR WALL, WHERE OCCURS

JOIST AT CORRIDOR 29

PLYW'D B.N., TYP.
JOIST W/ 'LUS' JOIST HANGER
BEAM
JOIST PERPENDICULAR TO BEAM
B.N.
JOIST W/ 'LUS' JOIST HANGER
BEAM
NOTE: PLYW'D SPLICE & CHANGE OF DIRECTION TO OCCUR AT CENTER OF BEAM
CHANGE JOIST DIRECTION AT BEAM

JOIST AT FLUSH BEAM 35

33

2x6 JST. @ 24" o.c. MAX.
'U' JST. HNG'R
ST22 STRAP EA. END OF BOX. COL.
FULL HT. & ALL SIDES OF BOX OUT COL.
SHEAR WALL PER PLAN
PROVIDE SOLID BLK'G AT STRAP WHEN NEEDED AT ENDS OF BOX COL. FOR STRAP INSTALLATION
SEE PLANS

FRAMING AT BOXED COLUMN 30

24

NOTE: PLYW'D SPLICE & CHANGE OF DIRECTION TO OCCUR AT CENTER OF BEAM
2x PLATE w/ 16d @ 12" o.c., SEE SHEAR WALL SCHED. FOR SILL PLATE NAILING AT SHEAR WALLS
E.N., TYP.
PLYW'D B.N., TYP.
2x BLK'G w/ 16d @ 3" o.c. EA.
FLR. TRUSS BY OTHERS
BEAM PER PLANS

CHANGE OF JOIST DIRECTION AT DROP BEAM 23

2x PLATE w/ 16d @ 12" o.c., SEE SHEAR WALL SCHED. FOR SILL PLATE NAILING AT SHEAR WALLS
E.N., TYP.
PLYW'D B.N., TYP.
FLR. TRUSS BY OTHERS
2x BLK'G w/ 16d @ 3" o.c. EA.
BEAM PER PLANS

JOIST PERPENDICULAR TO DROP BEAM 24

2x PLATE w/ 16d @ 12" o.c., SEE SHEAR WALL SCHED. FOR SILL PLATE NAILING AT SHEAR WALLS
E.N., TYP.
PLYW'D B.N., TYP.
FLR. TRUSS BY OTHERS
BEAM PER PLANS

22

NOTE: PLYW'D SPLICE & CHANGE OF DIRECTION TO OCCUR AT CENTER OF WALL
2x PLATE w/ 16d @ 12" o.c., SEE SHEAR WALL SCHED. FOR SILL PLATE NAILING AT SHEAR WALLS
2-2x BLK'G w/ 16d @ 3" o.c.
E.N., TYP.
FLR. TRUSS BY OTHERS
4x BLK'G
TRUSS, TYP.
24" @ "CS" STRAPS
36" @ "CMST" STRAP
STRAP LENGTH PER PLANS

CHANGE OF JOIST DIRECTION AT WALL 19

SHEAR WALL, WHERE OCCURS, SPLICE AT TOP PLATES
E.N., TYP. TO ALL PLATES
B.N., TYP.
2-2x BLK'G w/ 16d @ 3" o.c. EA.
2x TOP PLATES w/ 16d @ 4" o.c.
STL. BM. AT 13B
2x PLATE w/ 16d @ 12" o.c., SEE SHEAR WALL SCHED. FOR SILL PLATE NAILING AT SHEAR WALLS
1" CYPCRETE (MAX.), TYP. SEE ARCH. DWG. FOR SPEC.
FLR. TRUSS BY OTHERS
DROP BM. WHERE OCCURS
DBL. SIDE SHEAR WALL @ 13A
TOP PLATES
FLR. TRUSS BY OTHERS
FLR. TRUSS BY OTHERS
E.N., TYP.

NOTE:
1. SEE SHEAR WALL SCHEDULE FOR SILL PLATE NAILING AT SHEAR WALLS. WHERE PLYWOOD SHEATHING IS CONTINUED AND SPLICED AT TOP PLATES AND E.N. TO PLATES, SILL PLATE NAILING MAY BE INCREASED TO 16d @ 12" o.c.
2. FRAMING ANCHORS MAY BE OMITTED IF PLYWOOD SHEATHING IS CONTINUED AND SPLICED AT TOP PLATES.

JOIST PERPEND. TO EXTERIOR WALL 13

NOTE: PLYW'D SPLICE & CHANGE OF DIRECTION TO OCCUR AT CENTER OF BEAM
2x PLATE w/ 16d @ 12" o.c., SEE SHEAR WALL SCHED. FOR SILL PLATE NAILING AT SHEAR WALLS
E.N., TYP.
PLYW'D B.N., TYP.
BEAM PER PLANS
BOTT. CHORD BEARING FLR. TRUSS w/ 'THA' TRS. HNG'R TO BEAM, BY OTHERS
2x BLK'G

CHANGE OF JOIST DIRECTION AT FLUSH BEAM 20

SHEAR WALL, WHERE OCCURS, SPLICE AT TOP PLATES
E.N., TYP. TO ALL PLATES
2x PLATES w/ 16d @ 4" o.c.
3-10d PER BLK.
2x6 MIN. BLK. 48" o.c.
FLR. TRUSS BY OTHERS
DBL. SIDE SHEAR WALL @ 14A
FLUSH/DROP BM. WHERE OCCURS
24" MAX
2x PLATE w/ 16d @ 12" o.c., SEE SHEAR WALL SCHED. FOR SILL PLATE NAILING AT SHEAR WALLS

NOTE:
1. SEE SHEAR WALL SCHEDULE FOR SILL PLATE NAILING AT SHEAR WALLS. WHERE PLYWOOD SHEATHING IS CONTINUED AND SPLICED AT TOP PLATES AND E.N. TO PLATES, SILL PLATE NAILING MAY BE INCREASED TO 16d @ 12" o.c.
2. FRAMING ANCHORS MAY BE OMITTED IF PLYWOOD SHEATHING IS CONTINUED AND SPLICED AT TOP PLATES.

JOIST PARALLEL TO EXTERIOR WALL 14

2x PLATE w/ 16d @ 12" o.c., SEE SHEAR WALL SCHED. FOR SILL PLATE NAILING AT SHEAR WALLS
2-2x BLK'G w/ 16d @ 3" o.c.
E.N., TYP.
PLYW'D B.N., TYP.
BEAM PER PLANS
BOTT. CHORD BEARING FLR. TRUSS w/ 'THA' TRS. HNG'R TO BEAM, BY OTHERS
2x BLK'G

JOIST PERPENDICULAR TO FLUSH BEAM 21

2x PLATE w/ 16d @ 12" o.c., SEE SHEAR WALL SCHED. FOR SILL PLATE NAILING AT SHEAR WALLS
2-2x BLK'G w/ 16d @ 3" o.c.
E.N., TYP.
FLR. TRUSS BY OTHERS
TOP PLATES
DROP BEAM, WHERE OCCURS
E.N., TYP.
DBL. SIDE SHEAR WALL, WHERE OCCURS

JOIST PERPENDICULAR TO INTERIOR WALL 15

2-2x TOP PLATES MIN. (OPTION: 4-2x TOP PLATES MAY BE USED IF CONTRACTOR USES UNIFORM STUD LENGTHS FOR ALL WALLS, STITCH NAILED w/ 16d @ 12" o.c.)
2x PLATE w/ 16d @ 12" o.c., SEE SHEAR WALL SCHED. FOR SILL PLATE NAILING AT SHEAR WALLS
E.N., TYP.
3-10d PER BLK.
FLR. TRUSS BY OTHERS
2x6 MIN. BLK. @ 48" o.c.
DRAG TRUSS WHERE OCCURS PER PLAN
DBL. SIDE SHEAR WALL @ 16A
E.N., TYP.

JOIST PARALLEL TO INTERIOR WALL 16

NON-BRN'G WALL
2x PLATE w/ 2-16d PER BLK'G
3-10d PER BLK'G
2x4 LADDER BLK'G @ 48" o.c. w/ "22" CLIPS, E.E.
¼ GAP FILLER
FLR. TRUSS BY OTHERS, TYP.
CEILING
¾" CLR. MIN.
2x4 DIAG. BRACE & BLK'G @ 6'-0" o.c. MAX. AND AT WALL END WHERE LOWER WALL LENGTH EXCEEDS 12'-0"
SIMPSON DTC CLIP @ 48" o.c.
NON-BRNG WALL, WHERE OCCURS, SEE NOTE
STACKING WALLS
NON-STACKING WALLS
WALLS PARALLEL TO TRUSSES
WALLS PERPEND. TO TRUSSES
NOTE: MAINTAIN ¾" CLR. MIN. BETWEEN FLOOR JOIST AND NON-BEARING WALL

NON-BEARING WALL ABOVE FLOOR JOISTS 17

2x PLATE w/ 16d @ 12" o.c., SEE SHEAR WALL SCHED. FOR SILL PLATE NAILING AT SHEAR WALLS
2-2x BLK'G w/ 16d @ 3" o.c.
E.N., TYP.
PLYWOOD NAILING: 10d @ 4" o.c TO BEAM
PLYW'D B.N., TYP.
FLR. TRUSS BY OTHERS
TOP PLATES
BOTT. CHORD BEARING FLR. TRUSS w/ 'THA' TRS. HNG'R TO BEAM, BY OTHERS
2x BLK'G
2x PLATE w/ 16d @ 12" o.c. TO BM
FLUSH BEAM PER PLANS

JOIST PERPENDICULAR TO INTERIOR WALL 15

18

SIMPSON BC OR EPC POST. CAP, USE CCQ OR ECCQ COL CAP AT 12A
MSTC40 STRAP, FAR SIDE
BEAM (HEIGHT VARIES, SEE PLANS)
FRM'G ANCH. EA. SIDE, TOP & BOTT.
POST PER PLAN OR MULTIPLE STUDS – SPIKED TOGETHER w/ 16d @ 12" o.c.
CONDITION A (BEAM ABOVE PLATE)
CONDITION B (BEAM FLUSH w/ PLATE)
CONDITION C (BEAM BELOW PLATE)

BEAM TO COLUMN CONN. 11

FLUSH BEAM AT 9A
48" MIN. w/ 16d @ 1-3/4" o.c. AT CMST STRAP, U.N.O. ON PLANS
SEE PLANS FOR STRAP SIZE (MSTA36 MIN. IF NONE NOTED ON PLANS)
STRAP LOCATION AT 9B
DROP BEAM
3-16d FOR 6" DEEP HDR. ADD 1-16d FOR EA. ADD'L 2" IN DEPTH
SIMPSON AEC COL. CAP, U.N.O. ON PLANS. PROVIDE 'ECCQ' COL. CAP AT 9C
DBL. STUDS – SPIKED TOGETHER w/ 16d @ 12" o.c.
DBL. STUD OR POST PER PLAN. SEE PLANS FOR POST SIZE

PARALLEL DROP BEAM 9

A35 ANCH. TYP. @ 8'-0" OR LARGER OPN'G.
2-2x KING STUDS, USE 3-2X KING STUDS FOR OPN'GS 8'-0" AND LARGER, U.N.O.
BEAM/GIRDER TRUSS OR SQUASH BLK'G AT POST ABV.
POST
TRIMMER STUDS (MATCH BELOW)
SOLID BLK'G AT BEAM OR POST ABOVE
POST, ALIGN w/ BEAM OR FOST ABOVE
4-16d FOR 6" DEEP HDR. ADD 2-16d FOR EA. ADD'L 2" IN DEPTH
PROVIDE BLK'G AT MID HT OF STUDS 12'-0" AND TALLER
HEADER PER PLAN
16d @ 12" o.c.
"ACE" COL. CAP AT OPENINGS 8'-0" AND LARGER
2x TRIMMER STUD, USE 2-2x TRIMMERS WHERE HDR. IS 4x8/6x8 OR LARGER/PSL/GLB, SEE PLANS WHERE TRIMMER POST ARE REQ'D. (PROVIDE 3-2x TRIMMERS MINIMUM AT ALL HEADERS ON 1st FLOOR, U.N.O.)
HOLDOWN & POST, WHERE OCCURS PER PLAN
SHEAR WALL, WHERE OCCURS PER PLAN
2x WINDOW SILL w/ 2-16d TOE NAILS EA. END, USE 2-2x WINDOW SILLS AT OPENING 8'-0" AND LARGER

NOTE: PROVIDE 2x HORIZ. BLKG. BTWN. STUDS @ 10'-0" o.c. MAX & AS FIRE BLK'G. AT CEILING LINE.

ELEVATION VIEW
16d @ 6" o.c. TYP., AT SHEAR WALLS USE 16d AT SAME SPACING AS EN. OF SHEAR WALL USE 8 EN. SPACING WHERE SHEAR WALL OCCURS ON BOTH SIDES OF WALL
SHEAR WALL, WHERE OCCURS
E.N. TYP. AT SHEAR WALL
HOLDOWN, WHERE OCCURS
E.N., TYP
WALL ENDS / OPENING
WALL INTERSECTION
WALL CORNERS
PLAN VIEW

TYPICAL WALL FRAMING 1

4'-0" MIN.
16d @ 16" o.c., TYP.
STUD OR MULLION BELOW PLATE SPLICE, TYP.
12-16d EA. SIDE OF PLATE SPLICE OR SIMPSON MSTA36 STRAP., U.N.O.

TYP. PLATE SPLICE 2

2-16d
2x4 FLAT @ 48" o.c.
2-16d EA. END.
FLOOR JST. OR BOTT. CHORD OF TRUSS
1/2" CLR.
2x GWD. NAILER
PARALLEL TO FRAMING
1/2" CLR.
FLOOR JST. OR BOTT. CHORD OF TRUSS
SIMPSON DTC CLIP @ 48" o.c.
PERPENDICULAR TO FRAMING

NON-BEARING WALL FRAMING 3

3'-0" MAX. OPENING, UNO.
GIRDER TRS w/ VERT. WEB FOR HANGER OR BEAM (WHERE NOTED ON PLANS)
SECTION A–A
4x6 MIN. HEAD OUT BLK'G, UNO.
3'-0" MAX. OPENING, UNO.
SIMPSON 'W' HANGER, TYP., UNO.
SIMPSON 'BA' HANGER, UNO..

TYP. FRAMING AT OPENING 4

INTERSECTING BEAM
CONT. BEAM (w/ STRAP AT FAR SIDE WHERE BEAM SPLICE OCCURS)
BEAM SPLICE WHERE OCCURS
MSTA36 STRAP AT BEAM SPLICE, U.N.O. ON PLANS
SIMPSON CCC OR CCT COL. CAP
SIMPSON CCQ COL. CAP, USE ECCQ COL CAP AT BEAM END CONDITIONS, U.N.O.
POST, TYP.
INTERSECTING BEAMS
CONT. BEAMS
END CONDITION

BEAM PERPENDICULAR TO WALL 12

HEADER PER PLAN
OPENING
2x WINDOW SILL, 2-2x WINDOW SILL OR 4x SILL AT STRAP WIDER THAN 1 1/4"
SHEAR WALL TYP. PER PLAN ALL SIDES OF OPENING
HOLDOWN WHERE OCCURS PER PLAN
REMAINDER OF NOTES PER 1

SHEAR - ENTIRE FACE OF WALL (EFW) 5

POST OR MULTIPLE STUDS PER ATS SYSTEM MANUF.
HEADER PER PLANS
OPENING
2-2x PLATES AT WINDOW OPENINGS
PER ATS. MANUF.
SHEAR WALL, SEE PLANS FOR TYPE, SHEATH ALL SIDE OF OPENING AND ENTIRE FACE OF WALL
OMIT SECTION AT DOOR OPENINGS
ATS HOLDOWN SYSTEM, WHERE OCCURS, SEE PLANS FOR TYPE AND LOCATIONS., TYP.
REMAINDER OF NOTES PER 1

SHEAR WALL w/ ATS SYSTEM 6

REVISIONS:

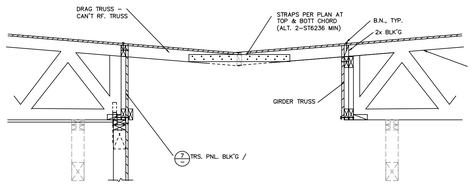
#	DATE	DESCRIPTION
1	9/12/19	PLAN CHK. #1

HCP ENGINEERING
2466 Redrock Drive
Corona, CA 92882-8605
Ph: (951) 738-0840
Fax: (951) 738-1432

FRAMING DETAILS

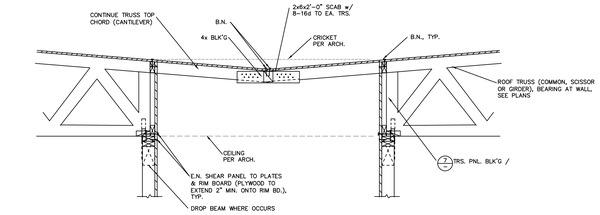
REGISTERED PROFESSIONAL ENGINEER HITESH C. PATEL No. C50389 Exp. 6-30-2019 CIVIL STATE OF CALIFORNIA
10/31/2020

PLOT DATE: 10/31/2020
SCALE:
DRAWN BY:
JOB NO. 2018-14
SHEET NO. SD-2



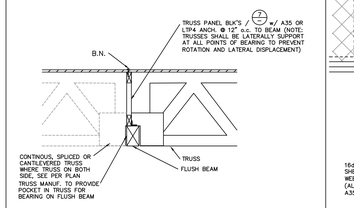
TRUSS TO WALL TRUSS TO GIRDER TRUSS

ROOF DRAG TRUSSES 31

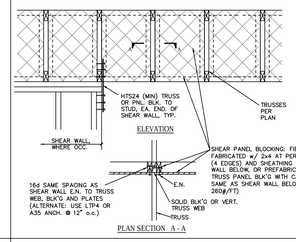


TRUSS TO WALL

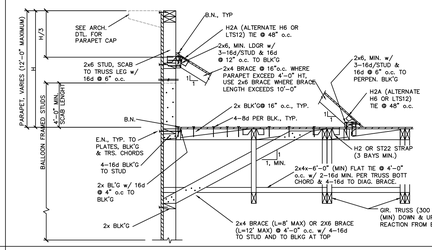
ROOF AT CORRIDOR 19



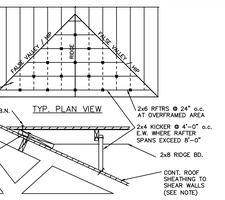
TRUSSES w/ POCKET AT FLUSH BEAM 13



TRUSS PANEL BLOCKING 7



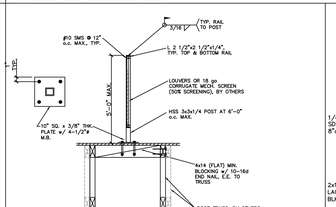
TRS PARALLEL TO EXT. WALL BALLOON w/ BRACED PARAPET 1



SECTION

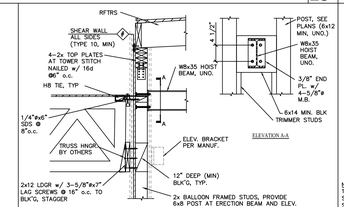
NOTE: (2) 2'–0" x 4'–0" MAX. PLYWOOD SECTIONS SHALL BE OMITTED BELOW OVERBUILT FRAMING FOR ACCESS AND VENTILATION. SPACE OPENINGS AS FAR APART AS POSSIBLE. BLOCK AND BOUNDRY NAIL SHEATHING AT ALL EDGES OF OPENING.

OVER-BUILT FRAMING (CALIF. FRAMING) 32

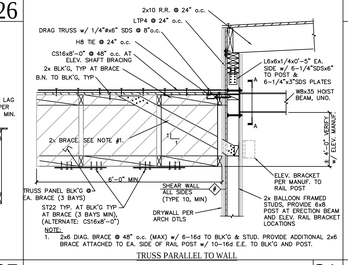


NOTE: ALL EXPOSED STEEL MEMBERS SHALL BE GALVANIZED

MECHANICAL SCREEN AT ROOF 26

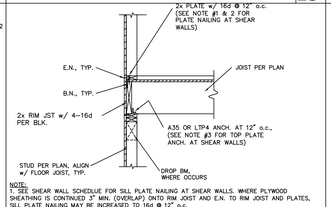


TRUSS PERPENDICULA TO WALL

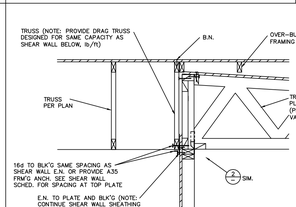


TRUSS PARALLEL TO WALL

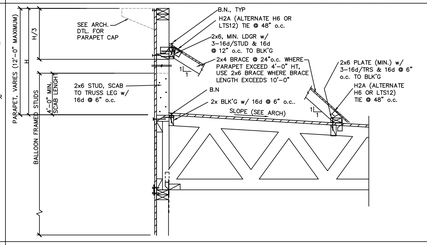
ROOF AT ELEV. SHAFT. (2HR WALL) 21



JOIST PERPEND TO EXT. WALL 14

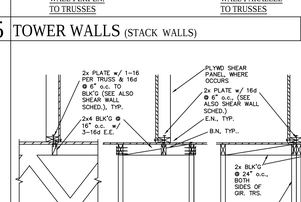


CHANGE OF TRUSS DIRECTION AT WALL 8

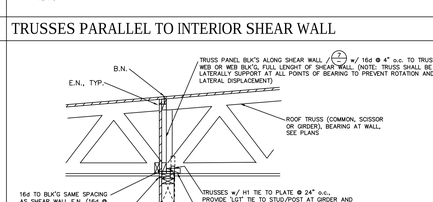


TRUSSES PERPEN TO EXT. WALL (w/ PARAPET LEG) 2

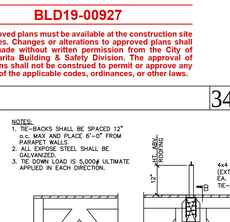
MECH. PLATFORM 27



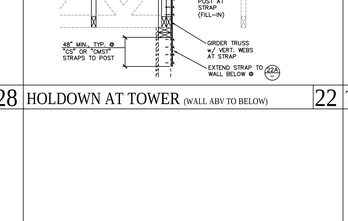
TOWER WALLS (STACK WALLS) 9



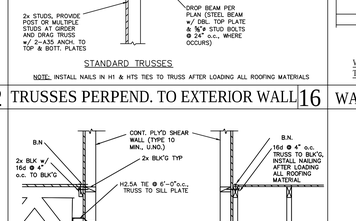
TRUSSES PARALLEL TO INTERIOR SHEAR WALL 3



FRAMING AT 2-HOUR FIRE SEPARATION 28



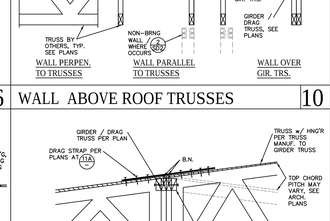
HOLDOWN AT TOWER (WALL ABV TO BELOW) 22



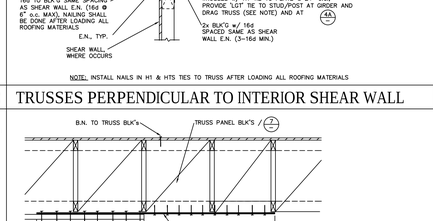
STANDARD TRUSSES

NOTE: INSTALL NAILS IN H1 & HTS TIES TO TRUSS AFTER LOADING ALL ROOFING MATERIALS

TRUSSES PERPEND. TO EXTERIOR WALL 16

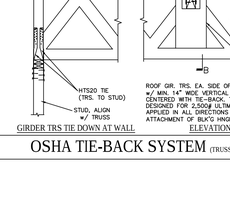


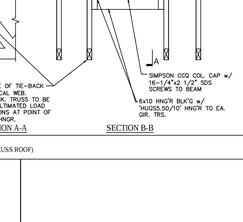
WALL ABOVE ROOF TRUSSES 10



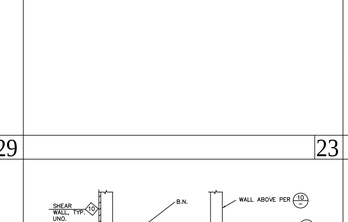
NOTE: INSTALL NAILS IN H1 & HTS TIES TO TRUSS AFTER LOADING ALL ROOFING MATERIALS

TRUSSES PERPENDICULAR TO INTERIOR SHEAR WALL 4

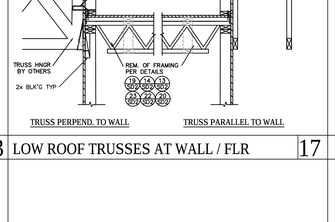




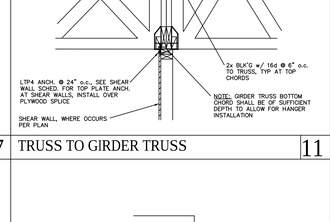
OSHA TIE-BACK SYSTEM (TRUSS ROOF) 29



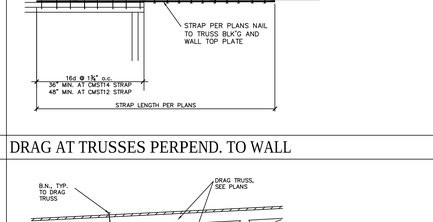
DRAG AT TRUSSES PERPEND. TO WALL 5



LOW ROOF TRUSSES AT WALL / FLR 17



TRUSS TO GIRDER TRUSS 11



DRAG AT TRUSSES PARALLEL TO WALL 6

ROOF AT ARCH 24

GIRDER TRS. UPLIFT CONN. 12

DRAG AT TRUSS PARALLEL TO WALL 6

REVISIONS:

1	9/12/19, PLAN CHK. #1	

HCP ENGINEERING
1307 West Sixth Street, Suite 208
Corona, CA 92882
Ph: (951) 738-0840
Fax: (951) 738-1432

ROOF FRAMING DETAILS

REGISTERED PROFESSIONAL ENGINEER HITESH C. PATEL No. C50389 Exp. 6–30–2019 CIVIL STATE OF CALIFORNIA
10/31/2020

PLOT DATE: 10/31/2020
SCALE:
DRAWN BY:
JOB NO. 2018–14
SHEET NO. SD-3

CITY OF SANTA CLARITA
BUILDING & SAFETY

APPROVED
Under SCMC Titles
18, 19, 20, 21, 24, 25
Nov 3, 2020
R. Abdel-Messih

BLD19-00927

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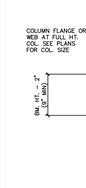
HCP ENGINEERING
1307 West Sixth Street, Suite 208
Corona, CA 92882
Ph: (951) 738-0840
Fax: (951) 738-1432

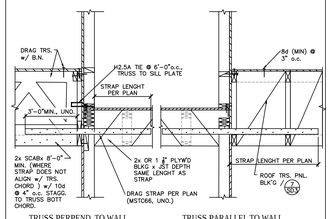
REVISIONS:

1	9/12/19, PLAN CHK. #1

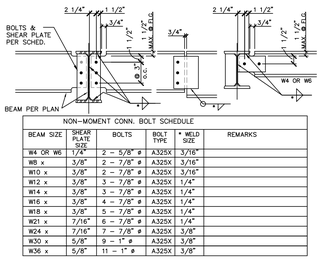
STEEL FRAMING DETAILS



WOOD BEAM TO STL COLUMN 12B



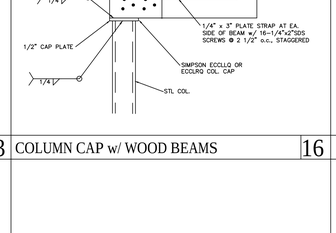
DRAG: LOW ROOF TRUSSES AT WALL / FLR 7



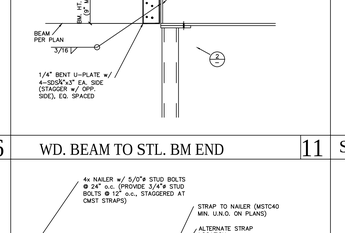
NON-MOMENT CONN. BOLT SCHEDULE

BEAM SIZE	SHEAR PLATE SIZE	BOLTS	BOLT TYPE	* WELD SIZE	REMARKS
W4 OR W6	1/4"	2 – 5/8" ø	A325X	3/16"	
W8 x	3/8"	2 – 7/8" ø	A325X	3/16"	
W10 x	3/8"	2 – 7/8" ø	A325X	3/16"	
W12 x	3/8"	3 – 7/8" ø	A325X	1/4"	
W14 x	3/8"	3 – 7/8" ø	A325X	1/4"	
W16 x	3/8"	4 – 7/8" ø	A325X	1/4"	
W18 x	3/8"	5 – 7/8" ø	A325X	1/4"	
W21 x	7/16"	6 – 7/8" ø	A325X	1/4"	
W24 x	7/16"	7 – 7/8" ø	A325X	3/8"	
W30 x	5/8"	9 – 1" ø	A325X	3/8"	
W36 x	5/8"	11 – 1" ø	A325X	3/8"	

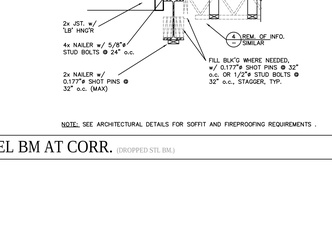
STEEL BEAM CONN. 1



SPECIAL BEAM HANGER 13

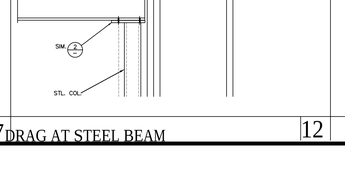


STEEL COLUMN TO WALL (AT END OF WALL) 8



BEAM TO COLUMN CONN. 2

'HGLTV' HNG'R TO STL. BM. 14



STEEL BEAM PERPERDICULAR TO WALL 9

NOTE: SEE ARCHITECTURAL DETAILS FOR SOFFIT AND FIREPROOFING REQUIREMENTS .

STEEL BEAM AT EXTERIOR WALL (DROPPED STL BM.) 3

HOLDOWN AT STL. BM. 15

DRAG BEAM AT STEEL BEAM (AT END OF WALL) 10

NOTE: SEE ARCHITECTURAL DETAILS FOR SOFFIT AND FIREPROOFING REQUIREMENTS .

JOIST AT STEEL BEAM (DROPPED STL BM.) 4

COLUMN CAP w/ WOOD BEAMS 16

WD. BEAM TO STL. BM END 11

NOTE: SEE ARCHITECTURAL DETAILS FOR SOFFIT AND FIREPROOFING REQUIREMENTS .

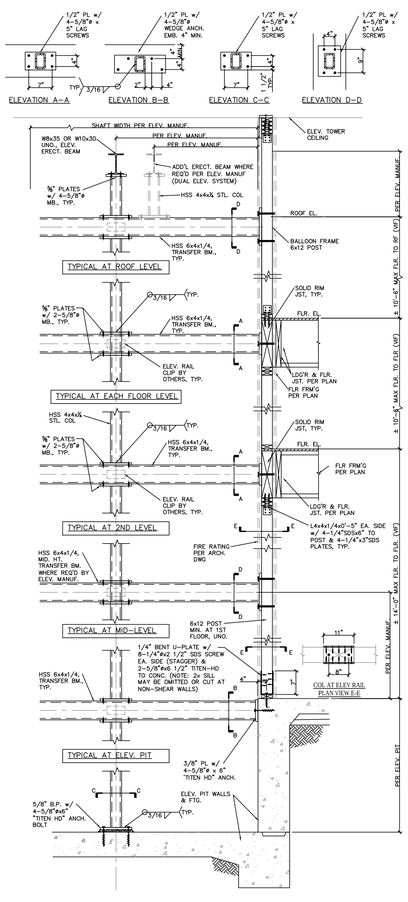
STEEL BM AT CORR. (DROPPED STL BM.) 5

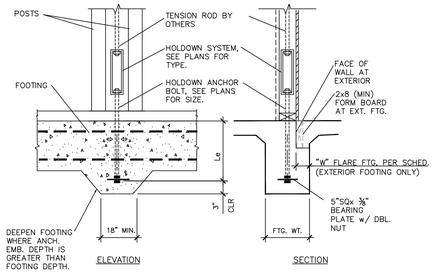
DRAG AT STEEL BEAM 12

REGISTERED PROFESSIONAL ENGINEER
WITESH C. PATEL
No. C50389
Exp. 6-30-2019
CIVIL
STATE OF CALIFORNIA

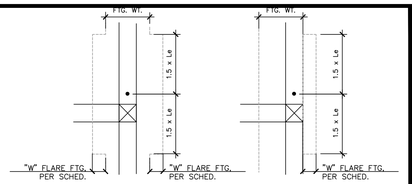
PLOT DATE: 10/31/2020
SCALE:
DRAWN BY:
JOB NO. 2018-14
SHEET NO. SD-4

CENTER RAIL STEEL COL. AT ELEVATOR 30

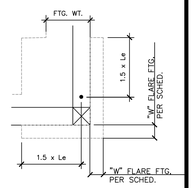




ELEVATION SECTION



INTERIOR FTG. PLAN VIEW EXTERIOR FTG. PLAN VIEW



CORNER PLAN VIEW

ANCHOR BOLT DIA. & TYPE		Le= MIN. EMBED. DEPTH (EMB.)	BEARING PLATE SIZE	CAPACITY (lbs) 100% (ASD)	CAPACITY (lbs) x1.4 (STRENGHT)	"W" FLARE FTG. AT BOLT (in)
R5	5/8" A36	12"	3.5	6,136	8,180	6"
R6	3/4" A36	12"	3.5	8,836	11,780	6"
R7	7/8" A36	16"	3.5	12,030	16,040	6"
R8	1" A36	18"	4.0	15,705	20,940	6"
R9	1 1/8" A36	21"	4.0	19,880	26,510	12"
R10	1 1/4" A36	24"	6.0	24,544	32,720	12"
R12	1 1/2" A36	27"	6.0	35,343	47,120	14"
R5 HS	5/8" A193-B7	18"	4.0	12,800	17,940	6"
R6 HS	3/4" A193-B7	21"	6.0	18,408	24,720	12"
R7 HS	7/8" A193-B7	24"	6.0	25,055	33,410	12"
R8 HS	1" A193-B7	27"	6.0	32,200	52,200	16"
R9 HS	1 1/8" A-193-B7	30"	6.0	47,000	65,800	16"
R10 HS	1 1/4" A193-B7	32"	8.0	59,600	83,500	16"
R12 HS	1 1/2" A-193-B7	36"	8.0	71,000	99,700	16"

⚠⚠ BEARING PLATE SIZE:
3.5 = 3½" SQ. x ⅜" THK.
4.0 = 4" SQ. x ⅜" THK.
6.0 = 6" SQ. x ½" THK.
8.0 = 8" SQ. x ½" THK.
12.0 = 12" SQ. x ¾"THK.

ANCHOR BOLTS AT SPECIAL HOLDOWN SYS.

SCALE: NONE

1

HOLDOWN	LEVEL	CUMULATIVE LOAD (KIPS) COMPRESSION	TENSION
H-A	Level 5:	6.0	5.5
H-D	Level 4:	15.1	14.0
H-H	Level 3:	26.3	24.6
H-I	Level 2:	39.1	36.7
H-K	Level 1:	57.1	53.8
H-Bs H-Cs	Level 5:	4.6	3.9
	Level 4:	11.7	8.4
	Level 3:	20.6	14.5
	Level 2:	46.9	23.2
	Level 1:	STL. BM	STL BM.
H-E H-L	Level 5:	3.7	3.0
	Level 4:	10.2	6.8
	Level 3:	18.6	12.5
	Level 2:	28.4	19.5
	Level 1:	42.5	30.4
H-F	Level 5:	3.6	3.0
	Level 4:	10.8	8.3
	Level 3:	20.4	16.2
	Level 2:	31.8	25.8
	Level 1:	48.3	40.1
H-1 H-5 H-7	Level 5:	5.5	3.8
	Level 4:	15.9	13.4
	Level 3:	29.8	26.5
	Level 2:	46.2	42.0
	Level 1:	69.8	64.5

ATS SYSTEM DESIGN LOADS

2

CITY OF SANTA CLARITA
BUILDING & SAFETY
APPROVED
Under SCMC Titles
18, 19, 20, 21, 24, 25
Nov 3, 2020
R. Abdel-Messih
BLD19-00927
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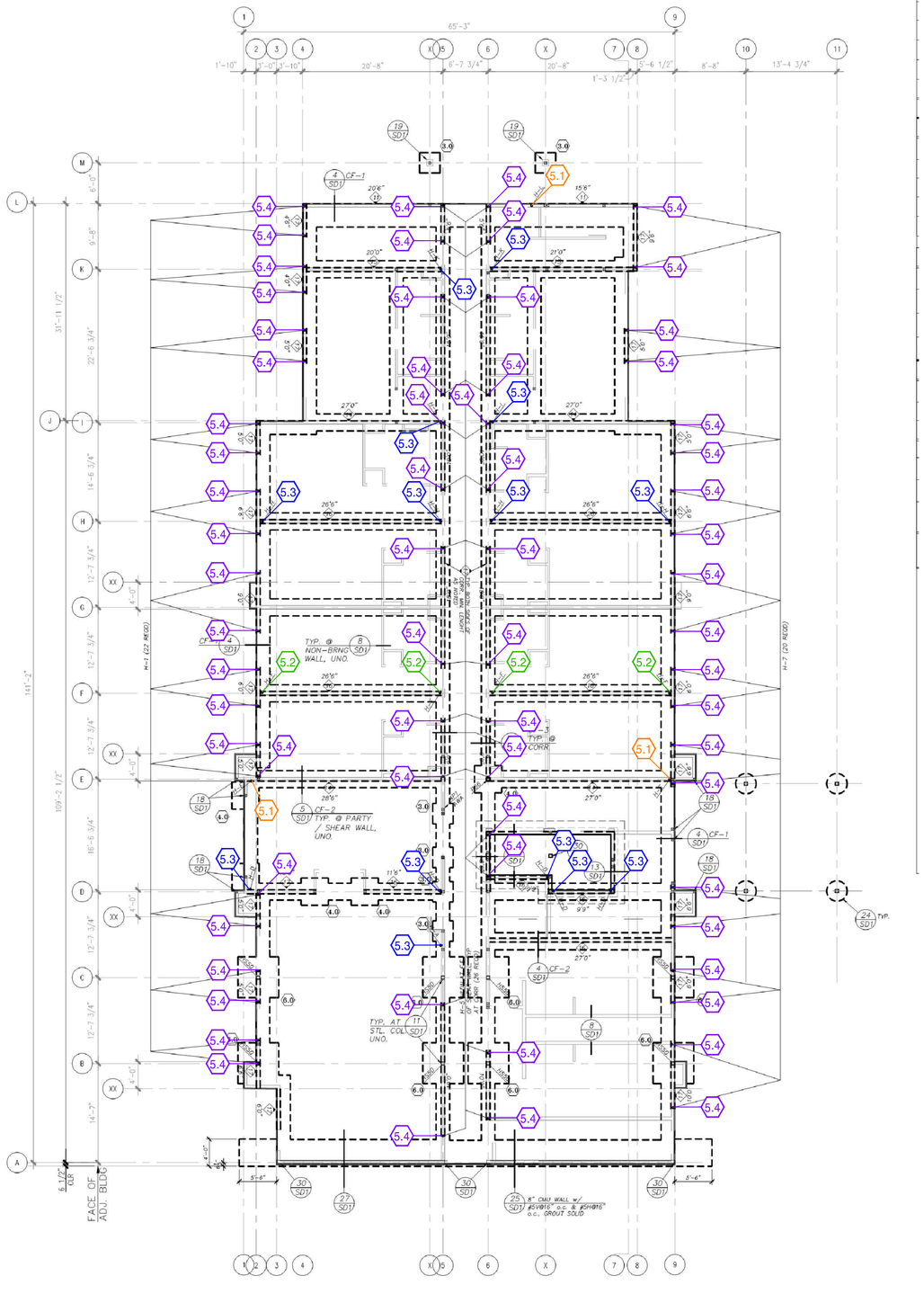
REVISIONS:
1 9/12/19, PLAN CHK. #1
2 12/06/19, PLAN CHK. #2

HCP ENGINEERING
2466 Redrock Dr.
Corona, CA 92882-8605
Ph: (951) 738-0840
Fax: (951) 738-1432

TYPICAL DETAILS

REGISTERED PROFESSIONAL ENGINEER HITESH C. PATEL No. C50389 Exp. 6-30-2019 CIVIL STATE OF CALIFORNIA
10/31/2020

ISSUE DATE: 2/26/16
SCALE:
PLOT DATE: October 31, 2020
JOB NO. 2018-14
SHEET NO. SD-5



1 FOUNDATION PLAN

SCALE 1/8"=1'-0"

CITY OF SANTA CLARITA
BUILDING & SAFETY

APPROVED

Under SCMC Titles
18, 19, 20, 21, 24, 25

Nov 3, 2020

R. Abdel-Messih

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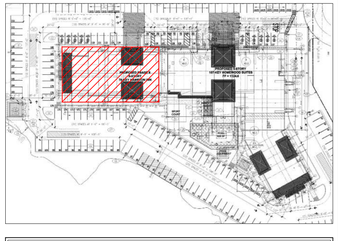
ATS RUN KEY	
RUN ID	REFERENCE
4.1	H-Bs, H-Cs
5.1	H-E, H-L
5.2	H-F
5.3	H-A, H-D, H-H, H-I, H-K
5.4	H-1, H-5, H-7

KEY PLAN



- ☑ Reviewed – No Exceptions Taken
- ☐ Reviewed – Make Corrections Noted
- ☐ Nonconforming – Revise and Resubmit
- ☐ Submit specified items

Submittal has been reviewed for general conformance to the design intent of the project and general conformance to the design intent of the project and general compliance with the information given in the contract documents. Any action shown is subject to the requirements of the plans and specifications. Contractor is responsible for all dimensions and shall confirm and correlate with field conditions; fabrication process and techniques of construction; coordination of its work with that of other trades; and the satisfactory performance of his work. Review of submittals and/or shop drawings does not relieve the contractor's responsibility for any errors, omissions and/or changes from the requirements of the contract documents, nor for errors and/or omissions made by the contractor and/or supplier in said submittal.

HCP ENGINEERING

By: HCP Date 12/06/2019

Job Name: HAMPTON INN, SANTA CLARITA

ATS LAYOUT GENERAL NOTES

1. THIS LAYOUT IDENTIFIES ATS RUNS WITH NAMES WHICH VARY FROM THOSE INDICATED ON THE STRUCTURAL DRAWINGS. ATS RUNS NOT SPECIFICALLY IDENTIFIED IN THIS LAYOUT SHALL BE INSTALLED WHERE NOTED ON THE STRUCTURAL DRAWINGS.
2. SIMPSON STRONG-TIE HAS NOT CONFIRMED AND IS NOT RESPONSIBLE FOR ANY OF THE DESIGN, ENGINEERING, CALCULATIONS OR DERIVATION OF STRUCTURAL FORCES RELATED TO THE BUILDING. ANY DESIGN OR DEMAND LOAD INFORMATION USED OR SHOWN ON THESE DRAWINGS WERE TAKEN FROM THE CONSTRUCTION DOCUMENTS AND HAVE NOT BEEN CONFIRMED BY SIMPSON STRONG-TIE.
3. REFER TO THE ATS DETAIL SHEETS FOR ROD LOCATIONS RELATIVE TO ENDWALL COMPRESSION POSTS.
4. THIS LAYOUT IS NOT FOR CONSTRUCTION WITHOUT SUBSTANTIATION OF REVIEW BY THE RESPONSIBLE ENGINEER OF RECORD AND THE GOVERNING BUILDING JURISDICTION.
5. THIS LAYOUT IS SPECIFIC TO ATS AND IS NOT APPLICABLE TO OTHER MANUFACTURER TIE DOWN SYSTEMS.
6. REFER TO THE ATS DETAIL SHEET FOR ADDITIONAL NOTES AND INFORMATION.

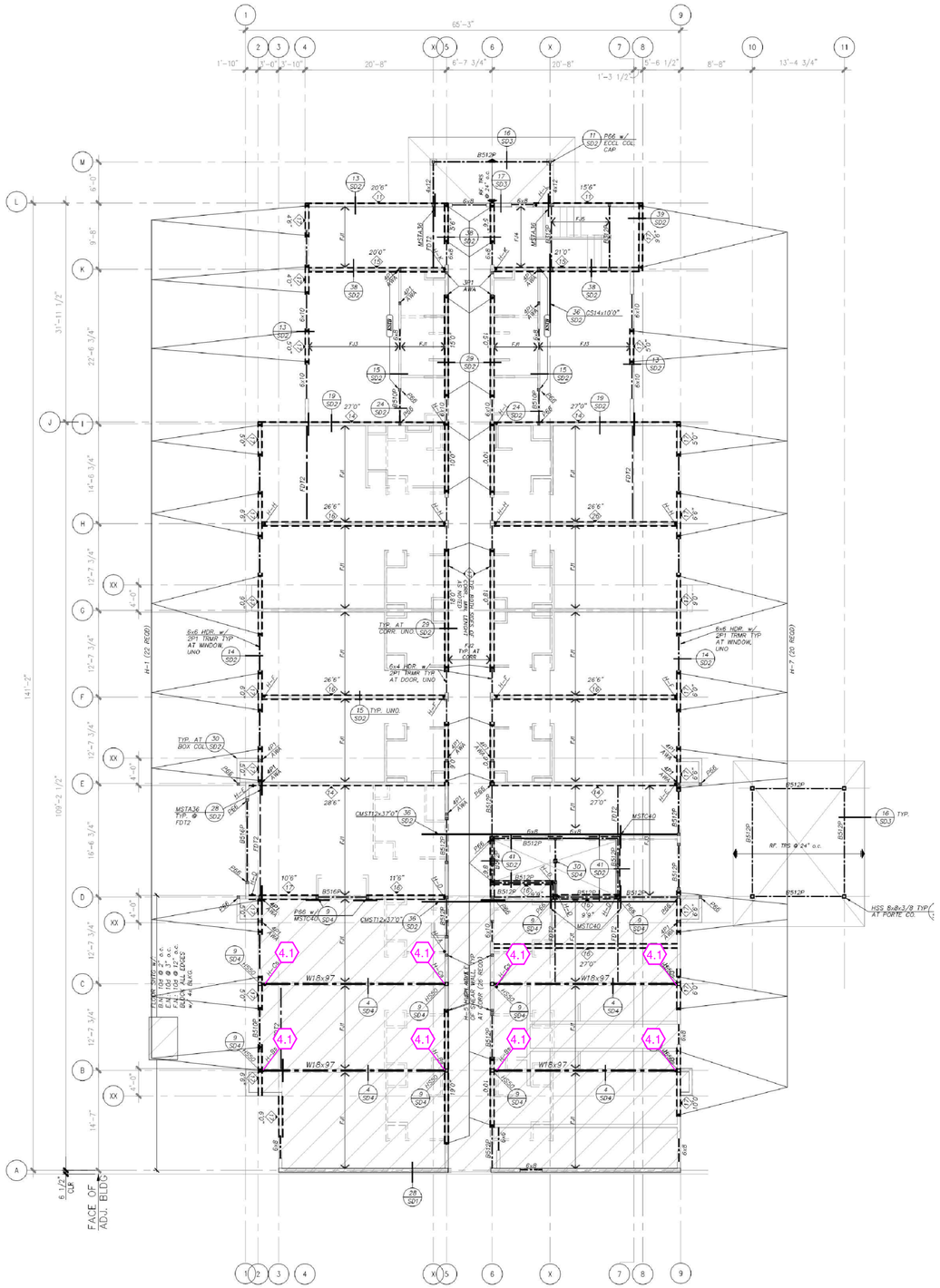
NO.	DATE	REVISIONS

SIMPSON STRONG-TIE, CO. INC.
• 5956 W. Las Positas Blvd.
Pleasanton, CA 94588
• Tel: (800) 999-5099
• Fax: (925) 847-15977
• Web site: www.strongtie.com
THERE IS NO EQUAL

DRAWINGS ARE BASED ON STRUCTURAL DRAWINGS DOCUMENTS BY:
HCP ENGINEERING
DATE OF PLANS (D.O.P.): 03/23/2019

HAMPTON INN
SANTA CLARITA, CA

NAME: H.NG.
DATE: 09/10/2019
SCALE: NO SCALE
SHEET:
OL-1
SHEET 1 OF 2
JOB NO. ES-194207



1 2ND FLOOR FRAMING PLAN

SCALE 1/8"=1'-0"

CITY OF SANTA CLARITA
BUILDING & SAFETY

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Under SCMC Titles
18, 19, 20, 21, 24, 25

Nov 3, 2020

R. Abdel-Messih

BLD19-00927

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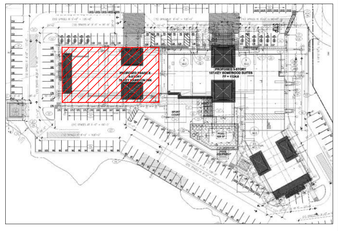
ATS RUN KEY	
RUN ID	REFERENCE
4.1	H-Bs, H-Cs
5.1	H-E, H-L
5.2	H-F
5.3	H-A, H-D, H-H, H-I, H-K
5.4	H-1, H-5, H-7

KEY PLAN



- ☑ Reviewed – No Exceptions Taken
- ☐ Reviewed – Make Corrections Noted
- ☐ Nonconforming – Revise and Resubmit
- ☐ Submit specified items

Submittal has been reviewed for general conformance to the design intent of the project and general conformance to the design intent of the project and general compliance with the information given in the contract documents. Any action shown is subject to the requirements of the plans and specifications. Contractor is responsible for all dimensions and shall confirm and correlate with field conditions; fabrication process and techniques of construction; coordination of its work with that of other trades; and the satisfactory performance of his work. Review of submittals and/or shop drawings does not relieve the contractor's responsibility for any errors, omissions and/or changes from the requirements of the contract documents, nor for errors and/or omissions made by the contractor and/or supplier in said submittal.

HCP ENGINEERING

By: *HCP* Date 12/06/2019

Job Name: *HAMPTON INN, SANTA CLARITA*

ATS LAYOUT GENERAL NOTES

1. THIS LAYOUT IDENTIFIES ATS RUNS WITH NAMES WHICH VARY FROM THOSE INDICATED ON THE STRUCTURAL DRAWINGS. ATS RUNS NOT SPECIFICALLY IDENTIFIED IN THIS LAYOUT SHALL BE INSTALLED WHERE NOTED ON THE STRUCTURAL DRAWINGS.
2. SIMPSON STRONG-TIE HAS NOT CONFIRMED AND IS NOT RESPONSIBLE FOR ANY OF THE DESIGN, ENGINEERING, CALCULATIONS OR DERIVATION OF STRUCTURAL FORCES RELATED TO THE BUILDING. ANY DESIGN OR DEMAND LOAD INFORMATION USED OR SHOWN ON THESE DRAWINGS WERE TAKEN FROM THE CONSTRUCTION DOCUMENTS AND HAVE NOT BEEN CONFIRMED BY SIMPSON STRONG-TIE.
3. REFER TO THE ATS DETAIL SHEETS FOR ROD LOCATIONS RELATIVE TO ENDWALL COMPRESSION POSTS.
4. THIS LAYOUT IS NOT FOR CONSTRUCTION WITHOUT SUBSTANTIATION OF REVIEW BY THE RESPONSIBLE ENGINEER OF RECORD AND THE GOVERNING BUILDING JURISDICTION.
5. THIS LAYOUT IS SPECIFIC TO ATS AND IS NOT APPLICABLE TO OTHER MANUFACTURER TIE DOWN SYSTEMS.
6. REFER TO THE ATS DETAIL SHEET FOR ADDITIONAL NOTES AND INFORMATION.

NO.	DATE	REVISIONS

SIMPSON STRONG-TIE, CO. INC.
• 5956 W. Las Positas Blvd.
Pleasanton, CA 94588
• Tel: (800) 999-5099
• Fax: (925) 847-15977
• Web site: www.strongtie.com
THERE IS NO EQUAL

DRAWINGS ARE BASED ON STRUCTURAL DRAWINGS DOCUMENTS BY: HCP ENGINEERING
DATE OF PLANS (D.O.P.): 03/23/2019

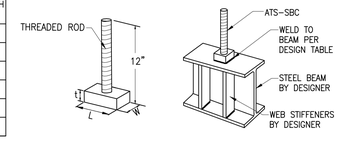
HAMPTON INN
SANTA CLARITA, CA

NAME: H.NG.
DATE: 09/10/2019
SCALE: NO SCALE
SHEET:
OL-2
SHEET 2 OF 2
JOB NO. ES-194207

1. ATS IS AN ASSEMBLAGE OF STEEL COMPONENTS, WHICH INCLUDE RODS, PLATES, COUPLER NUTS, TAKE-UP DEVICES AND NUTS. STUDS, POSTS AND BLOCKING ARE NOT SHIPPED WITH THE ANCHOR TIEDOWN SYSTEM. COMPRESSION MEMBERS SHOWN ARE THE MINIMUM SIZES BASED ON FULL SECTION MEMBERS, WITH THE SPECIFICATIONS NOTED BELOW, AND ARE BASED ON LOADS MATCHING THE TENSION FORCE OF THE ROD UNLESS SPECIFIC INSTRUCTIONS OR COMPRESSION FORCES ARE PROVIDED BY THE DESIGNER. THE DESIGNER IS RESPONSIBLE FOR COORDINATION OF LUMBER SPECIFICATIONS, STUD AND POST DESIGN DUE TO CODE LOAD COMBINATIONS AND MODIFICATIONS (HOLES, NOTCHES, ETC) TO THE STUDS AND POSTS. ALL COMPRESSION LUMBER IS BASED ON THE FOLLOWING, UNO: DFL
   2X4: No2
   2X6: No2
   3X4: NA
   3X6: NA
   4X: No1
   6X: No1
   ALL WOOD PLATES (TOP AND SOLE/SILL) ARE BASED ON THE FOLLOWING, UNO: DFL WITH F'cperp = 625 PSI
2. SIMPSON STRONG-TIE IS PROVIDING THE ANCHOR TIEDOWN SYSTEM TO MEET THE ATS MODEL SPECIFICATION OR DESIGN FORCES PROVIDED AND DETERMINED BY THE DESIGNER. SIMPSON STRONG-TIE HAS NOT CONFIRMED AND IS NOT RESPONSIBLE FOR ANY OF THE DESIGN, ENGINEERING, CALCULATIONS OR DERIVATION OF STRUCTURAL FORCES RELATED TO THE BUILDING. THE DESIGNER IS RESPONSIBLE FOR EVALUATING THE EFFECTS OF LUMBER SHRINKAGE AND ATS ELONGATION ON SHEARWALL DRIFT. SIMPSON STRONG-TIE HAS NOT CONFIRMED AND IS NOT RESPONSIBLE FOR VERIFYING THE SHEARWALL SYSTEM'S ADHERENCE TO BUILDING CODE DRIFT REQUIREMENTS OR ITS PERFORMANCE IN CONSIDERATION OF STRUCTURAL DEFORMATION COMPATIBILITY. THESE DRAWINGS ARE SPECIFIC TO ATS AND AREN'T APPLICABLE TO OTHER MANUFACTURER TIE DOWN SYSTEMS.
3. LUMBER SPECIFICATIONS BETWEEN THESE DRAWINGS AND THE PROJECT STRUCTURAL DRAWINGS MAY VARY AND RESULT IN LOWER ALLOWABLE COMPRESSION CAPACITIES. THE DESIGNER IS RESPONSIBLE FOR REVIEW AND ACCEPTANCE OF THE ATS INSTALLATION DRAWING LUMBER SPECIFICATIONS.
4. THE TAKE-UP DEVICE CONFIGURATIONS SHOWN ON THIS SHEET ARE FOR ILLUSTRATIVE PURPOSES ONLY. REFER TO THE ELEVATIONS ON THE SUBSEQUENT PAGES FOR TAKE-UP DEVICE CONFIGURATIONS BASED ON FRAMING CONDITIONS PARTICULAR TO EACH PROJECT OR PER DESIGNERS SHRINKAGE SPECIFICATIONS
5. CONTRACTOR OR INSTALLER OF ATS SHALL CUT RODS TO LENGTH AS REQUIRED.
6. DO NOT WELD PRODUCTS LISTED UNLESS THESE DRAWINGS SPECIFICALLY IDENTIFY A PRODUCT AS ACCEPTABLE FOR WELDING, OR UNLESS SPECIFIC APPROVAL FOR WELDING IS PROVIDED BY THE ENGINEER OF RECORD. SOME STEELS HAVE POOR WELDABILITY AND A TENDENCY TO CRACK WHEN WELDED. CRACKED STEEL WILL NOT CARRY LOAD AND MUST BE REPLACED.
7. FULLY ENGAGE EACH ROD INTO THE SPECIFIED COUPLING NUT OR CTUD UNTIL EACH ROD CAN BE SEEN FULLY IN THE WITNESS HOLES OR POSITIVE STOP HAS BEEN ACHIEVED.
8. INSTALL NUTS AND ISOLATOR NUTS SNUG TIGHT, PLUS AN ADDITIONAL 1/2 TURN
9. IN THE EVENT OF A DISCREPANCY BETWEEN THE STRUCTURAL DRAWINGS AND THESE INSTALLATION DRAWINGS, THE STRUCTURAL DRAWINGS ALWAYS GOVERN
10. SIMPSON STRONG-TIE HIGHLY RECOMMENDS THAT A PRE-CONSTRUCTION MEETING BE HELD PRIOR TO PLACEMENT OF CONCRETE TO ASSIST IN THE INSTALLATION PROCESS AND VERIFY QUANTITIES. TO COORDINATE THIS MEETING, CALL SIMPSON SALES AT 800-999-5099
11. PLANS ARE NOT FOR CONSTRUCTION WITHOUT SUBSTANTIATION OF REVIEW BY THE RESPONSIBLE DESIGNER AND THE GOVERNING BUILDING JURISDICTION.

1 GENERAL NOTES AND CONDITIONS OF USE

MODEL	ROD TO BEAM PLATE SIZE			FILLET WELD	
	L (IN)	W (IN)	t (IN)	SIZE (IN)	LENGTH (IN)
ATS-SBC5H	3	3	3/4	1/4	5
ATS-SBC6H	3	3	1	5/16	5
ATS-SBC8H	3	3	1 1/4	5/16	10
ATS-SBC10H	5	3	1 1/2	5/16	14
ATS-SBC11H	6	3	1 1/2	5/16	16
ATS-SBC12H	7	3	1 3/4	5/16	18

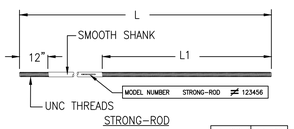


1. THE WELD LENGTH FOR THE ATS-SBC5H AND ATS-SBH6H REQUIRES ONLY TWO OPPOSING SIDES OF THE PLATE TO BE FILLET WELDED FULL LENGTH LESS A 1/4" STANDOFF AT CORNERS
2. THE WELD LENGTH FOR ATS-SBC8H UP TO THE ATS-SBC12H REQUIRES ALL FOUR SIDES OF THE PLATE TO BE FILLET WELDED WITH FULL LENGTH LESS A 1/4" STANDOFF AT CORNERS
3. ALL FILLET WELDS, F_{Exx}, TO BE GREATER THAN OR EQUAL TO 70 KSI AND TO FOLLOW GEOMETRY AND STANDARDS PER AISC AND AWS

2 STEEL BEAM CONNECTOR (SBC)

ROD DIA. (IN)	FULL THREAD MODEL NO.	STRONG-ROD MODEL NO.
MILD STEEL ROD		
1/2	ATS-R4 or ATR1/2	–
5/8	ATS-R5 or ATR5/8	–
3/4	ATS-R6 or ATR3/4	–
7/8	ATS-R7 or ATR7/8	–
1	ATS-R8 or ATR1	–
1 1/8	ATS-R9 or ATR1-1/8	–
1 1/4	ATS-R10 or ATR1-1/4	–
1 3/8	ATS-R11 or ATR1-3/8	–
1 1/2	ATS-R12 or ATR1-1/2	–
1 3/4	ATS-R14 or ATR1-3/4	–
2	ATS-R16 or ATR2	–
HIGH STRENGTH ROD		
5/8	ATS-HSR5	ATS-SR5H
3/4	ATS-HSR6	ATS-SR6H
7/8	ATS-HSR7	ATS-SR7H
1	ATS-HSR8	ATS-SR8H
1 1/8	ATS-HSR9	ATS-SR9H
1 1/4	ATS-HSR10	ATS-SR10H
1 3/8	ATS-HSR11	ATS-SR11H
1 1/2	ATS-HSR12	ATS-SR12H
1 3/4	ATS-HSR14	ATS-SR14H
2	ATS-HSR16	ATS-SR16H
1 1/8	ATS-HSSR9	–
1 1/4	ATS-HSSR10	–

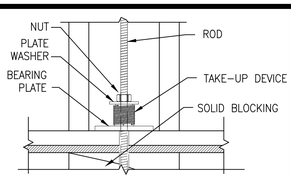
3 STRONG-ROD



STRONG-ROD

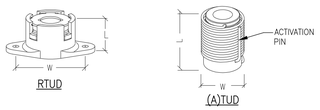
L (ft)	L1 (ft)
3 to 5	1
6 to 12	4

NOTES
1. FULLY THREADED AND STRONG-ROD IS UNCOATED OR PLAIN BLACK STEEL
2. ATS-R & ATS-SR STRONG-ROD MODEL DESIGNATION: THE NUMBERS FOLLOWING ATS-R(#) & ATS-SR(#) REPRESENTS THE ROD DIAMETER IN 1/8" INCREMENTS FOLLOWED BY LENGTH IN FT. (i.e. ATS-SR5H-10 IS 5/8" DIAMETER BY 10 FEET LONG HIGH STRENGTH ROD)
3. ATR MODEL DESIGNATION: THE NUMBERS FOLLOWING ATR(#) IS THE ROD ROD DIAMETER FOLLOWED BY LENGTH IN INCHES (i.e. ATR5/8X120 IS 5/8" DIAMETER BY 120 INCHES LONG MILD STEEL ROD)



MODEL NO.	MAXIMUM THREADED ROD SIZE (IN)	W (IN)	L (IN)	COMPENSATION (IN)
ATUD5	5/8	1 3/8	1 7/8	3/4
ATS-ATUD6-2	3/4	1 3/4	3 1/8	2
ATS-ATUD9	1 1/8	2 1/8	2 1/4	1
ATS-TUD9	1 1/8	2 1/8	2 1/4	1
ATS-ATUD9-2	1 1/8	2 1/8	3 7/8	2
ATS-ATUD9-3	1 1/8	2 1/8	5	3
ATS-TUD10	1 1/4	2 3/8	2 1/4	1
ATS-ATUD14	1 3/4	2 7/8	2 1/4	3/4
ATS-ATUD14-2	1 3/4	3	3 7/8	2
RTUD3B	3/8	2 1/4	1	NO LIMIT
RTUD4B	1/2	2 1/4	1	NO LIMIT
RTUD5	5/8	3 1/2	1 7/16	NO LIMIT
RTUD6	3/4	3 1/2	1 7/16	NO LIMIT

NOTES
1. CODE APPROVALS: ICC-ES ESR 2320; LA RR25643; FL 10007

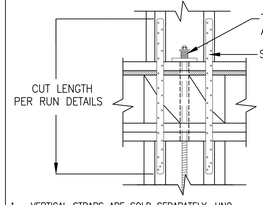


RTUD (A)TUD

INSTALLATION (TUD OR ATUD)
1. PLACE TAKE-UP DEVICE OVER ROD FROM LEVEL BELOW ONTO APPROPRIATE BEARING PLATE
2. INSTALL PLATE WASHER ON TOP OF TUD OR ATUD, AND SECURE THE NUT ON TOP BY FINGER TIGHTENING PLUS AN ADDITIONAL 1/2 TO 1/3 TURN WITH WRENCH
3. REMOVE ACTIVATION PIN

INSTALLATION (RTUD)
1. RTUD MUST BE INSTALLED ON TOP OF BPRTUD BEARING PLATE AND FASTENED THROUGH THE BPRTUD TO WOOD PLATES WITH:
   – RTUD 3B, 4B, 5 & 6: (2) #9X2-1/2 STRONG DRIVE® SD SCREWS

4 TAKE UP DEVICE (TUD)



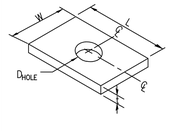
1. VERTICAL STRAPS ARE SOLD SEPARATELY, UNO.
2. WHEN STRAPS OCCUR WITH ATS, REFER TO THE STRUCTURAL DRAWINGS AND ATS INSTALLATION DETAILS.
3. REFER TO THE APPLICABLE CODE FOR MINIMUM NAIL EDGE AND END DISTANCES.

5 ALT. STRAP TRANSITION

MODEL	CORRESPONDING ROD DIAMETER
ATS-N4	1/2"
ATS-N5	5/8"
ATS-N6	3/4"
ATS-N7	7/8"
ATS-N8	1"
ATS-N9	1 1/8"
ATS-N10	1 1/4"
ATS-N11	1 3/8"
ATS-N12	1 1/2"
ATS-N14	1 3/4"
ATS-N16	2"

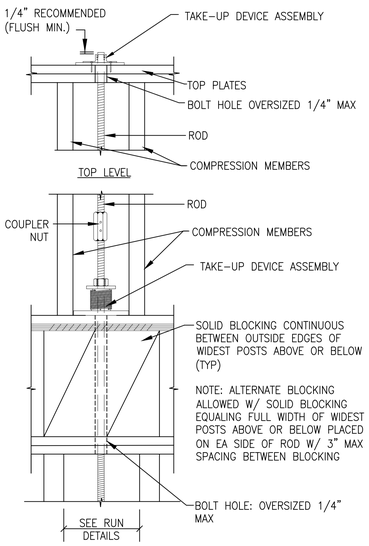
6 HEAVY HEX NUTS

MODEL	DIMENSIONS			
	W (in.)	L (in.)	t (in.)	D_{HOLE} (in.)
LBP 1/2	2	2	9/64	9/16
LBP 5/8	2	2	9/64	11/16
BP 3/4	2 3/4	2 3/4	5/16	13/16
BP 7/8	3	3	5/16	15/16
BP 1–3	3	3	3/8	1 1/16
BP 1–1/4	3	3	3/8	1 5/16
BP 1–1/2	3	3	3/8	1 9/16
BP 1–3/4	3	3	3/8	1 13/16



7 PLATE WASHERS

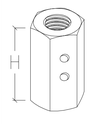
NOTE: NO SPLICE IN TOP PLATE IS PERMITTED WITHIN 8" OF ATS ROD



NOTES
1. SIMPSON STRONG-TIE ASSUMES 6" MINIMUM ANCHOR BOLT PROJECTION TO DETERMINE ROD LENGTHS ABOVE CONCRETE SURFACE UNLESS NOTED OTHERWISE BY THE DESIGNER. IF THIS VARIES, COORDINATE WITH SIMPSON STRONG-TIE PRIOR TO SHIPMENT OF RODS

8 INSTALLATION DETAIL

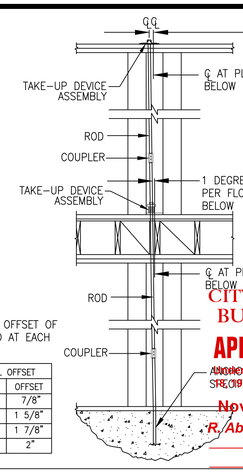
MODEL		ROD DIA (in.)	H^3 (in.)
MILD STEEL	HIGH STRENGTH		
CNW 1/2	–	1/2 TO 1/2	1 1/2
CNW 5/8	ATS-HSC55	5/8 TO 5/8	1 3/4
CNW 3/4	HSCNW3/4	3/4 TO 3/4	2
CNW 7/8	ATS-HSC77	7/8 TO 7/8	2 1/4
CNW 1	ATS-HSCW1	1 TO 1	2 1/2
ATS-C99	ATS-HSC99	1 1/8 TO 1 1/8	2 3/4
ATS-C1010	ATS-HSC1010	1 1/4 TO 1 1/4	3
ATS-C1111	ATS-HSC1111	1 3/8 TO 1 3/8	3 1/4
ATS-C1212	ATS-HSC1212	1 1/2 TO 1 1/2	3 1/2
ATS-C1414	ATS-HSC1414	1 3/4 TO 1 3/4	4
ATS-C1616	ATS-HSC1616	2 TO 2	4 1/2
CNW 5/8–1/2	ATS-HSC54	5/8 TO 1/2	1 5/8
CNW 3/4–5/8	ATS-HSC65	3/4 TO 5/8	1 7/8
ATS-C76	ATS-HSC76	7/8 TO 3/4	2 1/8
ATS-C87	ATS-HSC87	1 TO 7/8	2 3/8
ATS-C98	ATS-HSC98	1 1/8 TO 1	2 5/8
ATS-C109	ATS-HSC109	1 1/4 TO 1 1/8	2 7/8
ATS-C1110	ATS-HSC1110	1 3/8 TO 1 1/4	3 1/8
ATS-C1211	ATS-HSC1211	1 1/2 TO 1 3/8	3 3/8
ATS-C1412	ATS-HSC1412	1 3/4 TO 1 1/2	3 3/4
ATS-C1614	ATS-HSC1614	2 TO 1 3/4	4 1/4



Units in 1/8" Increments (Ex: 9 = 9/8 OR 1 1/8")
ATS-HSC98 — 2ND DIAMETER — 1ST DIAMETER
GRADE: C OR CNW = MILD STEEL; HSC OR HSCNW = HIGH STRENGTH

COUPLER NOTES AND INSTALLATION NOTES
1. TIGHTEN THE TWO RODS UNTIL EACH ROD CAN BE FULLY SEEN IN THE WITNESS HOLE
2. ADDITIONAL COUPLER NUTS SIZES INCLUDING OVERSIZED THREADS (OST) FOR GALVANIZED ANCHOR BOLTS ARE AVAILABLE – REFER TO NOMENCLATURE BREAKDOWN IN THIS DETAIL FOR ITEMS NOT LISTED IN TABLE
3. HEIGHTS LISTED ARE MINIMUM DIMENSIONS – FOR COUPLERS NOT LISTED MINIMUM HEIGHT = (1ST DIAMETER + 2ND DIAMETER + 1/2")
4. CNW AND HSCNW MODELS LISTED ABOVE AS APPLICABLE – ROD DIAMETER FOR CNW AND HSCNW MODELS ARE LISTED IN INCHES

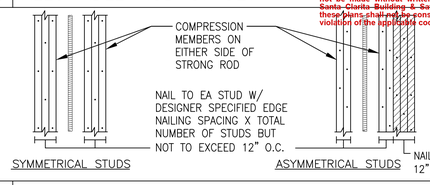
9 COUPLER NUTS



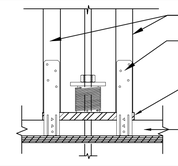
NOTE:
A 1 DEGREE MAXIMUM OFFSET OF THE ROD IS PERMITTED AT EACH LEVEL

ALLOWABLE HORIZONTAL OFFSET	
PLATE HEIGHT	OFFSET
4'-4" (BRIDGE BLOCK)	7/8"
8'-0"	1 5/8"
9'-0"	1 7/8"
10'-0" AND GREATER	2"

10 ALLOWABLE ROD OFFSET

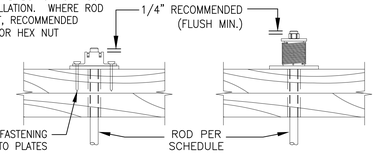


11 SHEARWALL EDGE NAILING

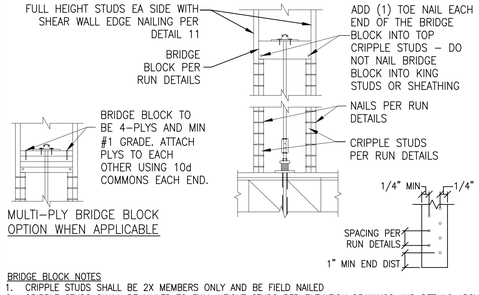


12 OPTIONAL SSP WHEN STUD IS OVER PLATE

MINIMUM OF 2 ½" CLEARANCE REQUIRED BETWEEN TOP OF ROD AND BOTTOM OF ROOF DECK AT TIME OF INSTALLATION. WHERE ROD IS REQUIRED TO BE CUT, RECOMMENDED 1/4" ABOVE THE RTUD OR HEX NUT

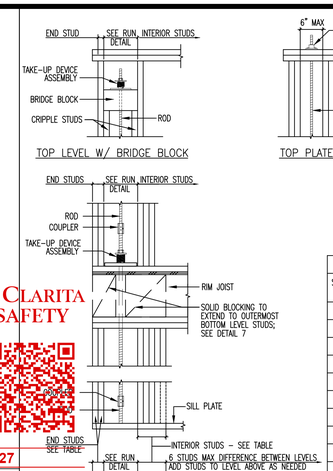


13 TAKE-UP DEVICE INSTALLATION AT TOP LEVEL



BRIDGE BLOCK NOTES
1. CRIPPLE STUDS SHALL BE 2X MEMBERS ONLY AND BE FIELD NAILED
2. CRIPPLE STUDS SHALL BE NAILED TO FULL HEIGHT STUDS PER ELEVATION DRAWINGS AND DETAILS ABOVE. IF (2) CRIPPLES ARE REQUIRED ON EACH SIDE OF ROD, NAIL CRIPPLE ADJACENT TO FULL HEIGHT STUD WITH SCHEDULED NAILS AND NAIL INNER CRIPPLE WITH ½ OF THE SCHEDULED NAILS. IF (3) CRIPPLES ARE REQUIRED ON EACH SIDE OF THE ROD, NAIL CRIPPLE ADJACENT TO FULL HEIGHT STUD WITH SCHEDULED NAILS AND THE MIDDLE CRIPPLE WITH ⅔ OF THE SCHEDULED NAILS AND THE INNER MOST CRIPPLE WITH ⅓ OF THE SCHEDULED NAILS
3. CRIPPLE STUDS MAY BE NAILED TO FULL HEIGHT STUDS FROM EITHER SIDE
4. IF BRIDGE BLOCK IS SOLID SAWN, IT MUST BE A MINIMUM #1 GRADE; SCL IS ALSO ACCEPTABLE
5. FOR SDS SCREW OPTIONS, CONTACT SIMPSON STRONG-TIE
6. ROD LENGTH AT BRIDGE BLOCK LEVEL BASED ON 52" HEIGHT ABOVE SUBFLOOR. ADDITIONAL ROD LENGTH WILL BE REQUIRED FOR BRIDGE BLOCK HEIGHTS ABOVE 52"

14 BRIDGE BLOCK DETAIL

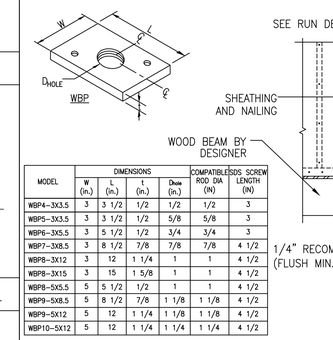


ASYMMETRICAL STUD CONVERSION		
SYMMETRICAL LAYOUT	ASYMMETRICAL CONFIGURATION	
	END OF WALL	INTERIOR SIDE
(1) 2x4	(2) 2x4	(1) 2x4
(2) 2x4	(2) 2x4	(2) 2x4
(3) 2x4	(2) 2x4	(4) 2x4
(4) 2x4	(2) 2x4	(6) 2x4
(5) 2x4	(2) 2x4	(8) 2x4
(6) 2x4	(2) 2x4	(10) 2x4
(7) 2x4	(2) 2x4	(12) 2x4
(8) 2x4	(2) 2x4	(14) 2x4
(9) 2x4	(2) 2x4	(16) 2x4
(10) 2x4	(2) 2x4	(18) 2x4
(11) 2x4	(2) 2x4	(20) 2x4
(12) 2x4	(2) 2x4	(22) 2x4

NOTES:
1. SYMMETRICAL LAYOUT CONFIGURATIONS ARE THE NUMBER OF COMPRESSION MEMBERS REQUIRED ON EACH SIDE OF ROD.
2. 2x6 MEMBERS SIMILAR

ASYMMETRICAL COMPRESSION MEMBER RULES
1. MAXIMUM SPACING BETWEEN END STUDS AND INTERIOR STUDS SHALL NOT EXCEED SPACING SPECIFIED IN JOB SPECIFIC RUN DETAILS
2. MAXIMUM SPACING BETWEEN END STUDS AND INTERIOR STUDS SHALL NOT EXCEED 6" WHERE A FLOOR SYSTEM WITH A RIM JOIST OR RIM BLOCKING IS NOT PRESENT (LEDGER OR TOP CHORD BEARING TRUSS CONDITION)
3. TOP PLATE AND STRAP TERMINATIONS SHALL USE TWO END STUDS ALL LEVELS
4. BRIDGE BLOCK TERMINATIONS SHALL USE TWO END STUDS AT THE UPPERMOST LEVEL WHEN A SINGLE CRIPPLE IS SPECIFIED AND SHALL USE ONE END STUD IF A DOUBLE CRIPPLE IS SPECIFIED (SEE JOB SPECIFIC RUN DETAILS FOR CRIPPLE SPECIFICATIONS); ALL OTHER LEVELS SHALL USE TWO END STUDS
5. ADDITIONAL LUMBER SHOULD BE ADDED AT THE UPPERMOST LEVEL AS NEEDED FOR END OF WALL FRAMING
6. A MINIMUM OF ONE INTERIOR STUD IS REQUIRED AT ALL LEVELS
7. AT ANY FLOOR LEVEL, A MAXIMUM OF 6 ADDITIONAL STUDS MAY BE USED AT THE INTERIOR STUDS AS COMPARED TO THE INTERIOR STUD PACK ABOVE
8. SEE DETAIL 11 FOR SHEAR WALL EDGE NAILING

15 ASYMMETRICAL COMPRESSION MEMBER DESIGN



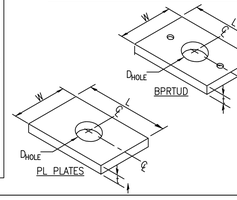
MODEL	DIMENSIONS				COMPATIBLE ROD DIA (IN)	SDS SCREW LENGTH (IN)
	W (in.)	L (in.)	t (in.)	D_{HOLE} (in.)		
WBP4–3X3.5	3	3 1/2	1/2	1/2	1/2	3
WBP5–3X3.5	3	3 1/2	1/2	5/8	5/8	3
WBP6–3X5.5	3	5 1/2	1/2	3/4	3/4	3
WBP7–3X8.5	3	8 1/2	7/8	7/8	7/8	4 1/2
WBP8–3X12	3	12	1 1/4	1	1	4 1/2
WBP8–3X15	3	15	1 5/8	1	1	4 1/2
WBP8–5X5.5	5	5 1/2	1/2	1	1	4 1/2
WBP9–5X8.5	5	8 1/2	7/8	1 1/8	1 1/8	4 1/2
WBP9–5X12	5	12	1 1/4	1 1/8	1 1/8	4 1/2
WBP10–5X12	5	12	1 1/4	1 1/4	1 1/4	4 1/2

NOTES:
1. THREAD ROD THROUGH WBP PLATE
2. THREAD HEX NUT ONTO ROD AND TIGHTEN AGAINST WBP PLATE
3. FASTEN ROD AND WBP PLATE ASSEMBLY TO WOOD BEAM WITH (2) SDS SCREWS (PROVIDED AS A KIT); SEE TABLE FOR SDS SCREW LENGTH

16 WOOD BEAM BEARING PLATE

MODEL	DIMENSIONS				COMPATIBLE TAKE-UP DEVICE SERIES
	W (in.)	L (in.)	t (in.)	D_{HOLE} (in.)	
PL5–3X3.5	3	3 1/2	3/8	11/16	ATUD/TUD
PL5–3X5.5	3	5 1/2	1/2	11/16	
PL6–3X3.5	3	3 1/2	3/8	13/16	
PL6–3X5.5	3	5 1/2	1/2	13/16	
PL9–3X5.5	3	5 1/2	1/2	1 3/16	
PL9–3X8.5	3	8 1/2	7/8	1 3/16	
PL14–3X8.5	3	8 1/2	7/8	1 13/16	
PL9–3X12	3	12	1 1/4	1 3/16	
PL14–3X12	3	12	1 1/4	1 13/16	
PL9–3X15	3	15	1 1/2	1 3/16	
PL10–3X15	3	15	1 1/2	1 5/16	
PL14–3X15	3	15	1 1/2	1 13/16	
PL9–5X5.5	5	5 1/2	1/2	1 3/16	
PL14–5X5.5	5	5 1/2	1/2	1 13/16	
PL9–5X8.5	5	8 1/2	7/8	1 3/16	
PL14–5X8.5	5	8 1/2	7/8	1 13/16	
PL9–5X12	5	12	1 1/4	1 3/16	
PL10–5X12	5	12	1 1/4	1 5/16	

MODEL	DIMENSIONS				COMPATIBLE TAKE-UP DEVICE SERIES
	W (in.)	L (in.)	t (in.)	D_{HOLE} (in.)	
BPRTUD3–4	3	3	1/4	5/8	RTUD
BPRTUD3–4B	3	3 1/2	1/4	5/8	
BPRTUD5–6A	3	4 1/2	1/4	1	
BPRTUD5–6B	3	5 1/2	1/2	1	
BPRTUD5–6C	3	7 1/2	3/4	1	



17 BEARING PLATES FOR ATUD/TUD AND RTUD

REVISIONS

NO.	DATE	REVISIONS

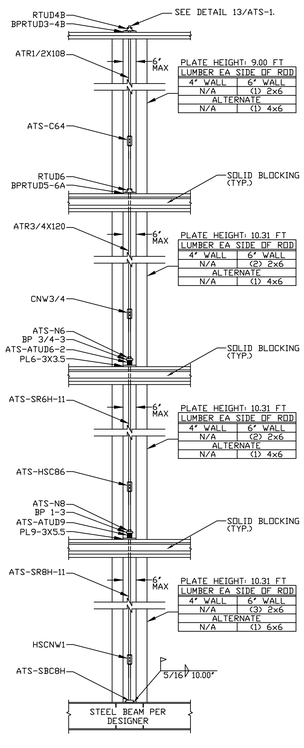
SIMPSON STRONG-TIE, CO. INC.
5956 W. Las Positas Blvd.
Pleasanton, CA 94588
Tel: (800) 999-5099
Fax: (925) 847-1597
Web site: www.strongtie.com
SIMPSON Strong-Tie
THERE IS NO EQUAL

DRAWINGS ARE BASED ON STRUCTURAL DRAWINGS
DOCUMENTS BY: HCP ENGINEERING
DATE OF PLANS (D.O.P.): 03/23/2019

HAMPTON INN
SANTA CLARITA, CA

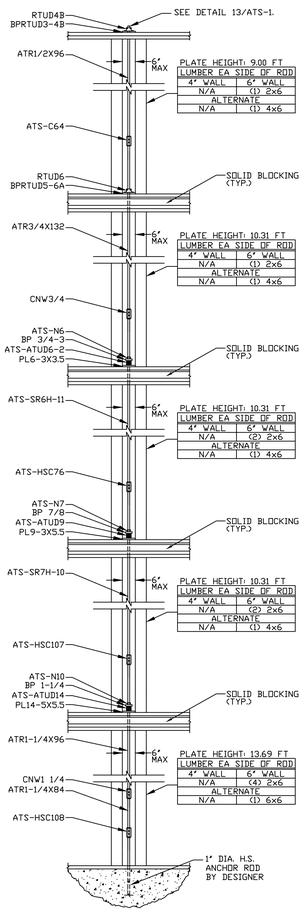
NAME: H.NG.
DATE: 09/10/2019
SCALE: NO SCALE
SHEET:
ATS-1
SHEET 1 OF 2
JOB NO. ES–194207

LEVEL	CUMULATIVE TENSION (kips)		INCREMENTAL TENSION (kips)	
	DEMAND	CAPACITY	DEMAND	CAPACITY
Level 5:	3.90	4.27	3.90	4.27
Level 4:	8.10	9.61	4.20	7.06
Level 3:	13.80	16.33	5.70	6.72
Level 2:	22.00	26.35	8.20	10.03



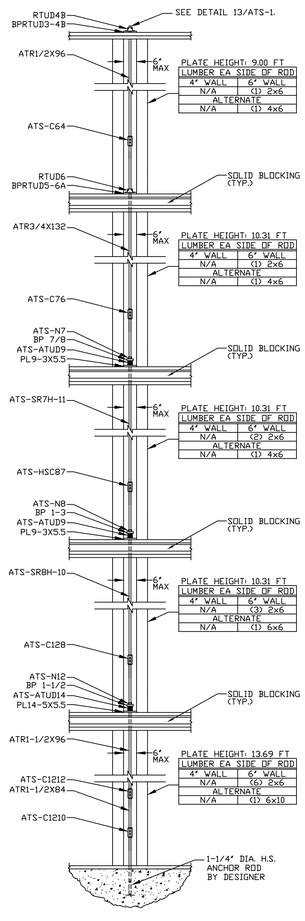
4.1
8 RUN(S)

LEVEL	CUMULATIVE TENSION (kips)		INCREMENTAL TENSION (kips)	
	DEMAND	CAPACITY	DEMAND	CAPACITY
Level 5:	3.40	4.27	3.40	4.27
Level 4:	7.80	9.61	4.40	7.06
Level 3:	12.80	16.33	5.00	6.72
Level 2:	18.30	26.35	5.50	10.03
Level 1:	25.90	26.69	7.60	16.26



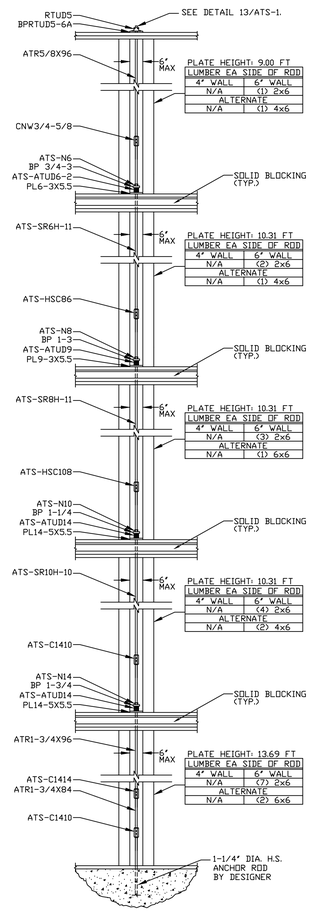
5.1
3 RUN(S)

LEVEL	CUMULATIVE TENSION (kips)		INCREMENTAL TENSION (kips)	
	DEMAND	CAPACITY	DEMAND	CAPACITY
Level 5:	2.80	4.27	2.80	4.27
Level 4:	7.60	9.61	4.80	7.06
Level 3:	14.70	19.63	7.10	10.03
Level 2:	23.50	29.66	8.80	10.03
Level 1:	36.50	38.44	13.00	16.26



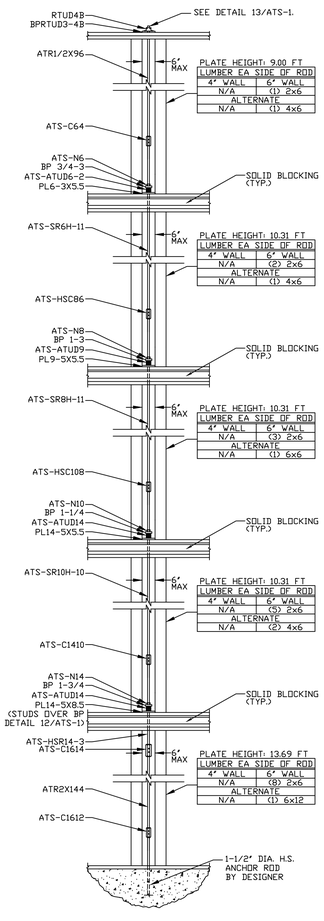
5.2
4 RUN(S)

LEVEL	CUMULATIVE TENSION (kips)		INCREMENTAL TENSION (kips)	
	DEMAND	CAPACITY	DEMAND	CAPACITY
Level 5:	5.60	6.67	5.60	6.67
Level 4:	13.40	17.16	7.80	10.49
Level 3:	23.10	27.19	9.70	10.03
Level 2:	34.70	43.45	11.60	16.26
Level 1:	50.60	52.31	15.90	16.26



5.3
14 RUN(S)

LEVEL	CUMULATIVE TENSION (kips)		INCREMENTAL TENSION (kips)	
	DEMAND	CAPACITY	DEMAND	CAPACITY
Level 5:	3.60	4.27	3.60	4.27
Level 4:	12.50	14.76	8.90	10.49
Level 3:	24.50	30.32	12.00	15.56
Level 2:	38.80	46.58	14.30	16.26
Level 1:	59.60	68.33	20.80	24.39



5.4
68 RUN(S)

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HAMPTON INN
SANTA CLARITA, CA

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DATE: 09/10/2019
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SHEET:
ATS-2
SHEET 2 OF 2
JOB NO. ES-194207